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Development at St Claires Villas Lucan

Additional Information
Planning Ref: SD22A/0372
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ONCE Civil & Structural Ltd

INTRODUCTION

This report has been prepared in reply to South Dublin County Council REQUEST FOR ADDITIONAL INFORMATION, decision order No.1470, register reference SD22A/0372

5. (i) The Applicant is requested to submit a drawing showing additional surface water attenuation for proposed development. Additional attenuation shall be by means of SuDS (Sustainable Drainage Systems).

The Site Drainage Layout Drawing No.5845/01 rev C shows the proposed surface water layout. Attenuation provided implementing Suds features green roof and permeable paving

- (ii) The applicant is required to submit a revised drawing and report showing additional surface water attenuation provided by means of SuDS (Sustainable Drainage Systems) to include:
- a) Above ground natural multifunctional (amenity, biodiversity, water treatment/quality and attenuation) sustainable natural drainage solutions such as blue/green roofs, permeable pavement, bioretention areas, rain gardens, filter drains, swales, bioretention tree pits.
- b) Demonstrate the biodiversity value of SuDS especially important given the site is in a Primary Green Corridor and next to a Core area.
- c) Existing and modified flows.
- d) Detailed design of SUDs features showing how they work.
- e) A comprehensive SUDS management Plan to demonstrate that the proposed SUDS features

have reduced the rate of run off into the existing surface water drainage.

f) Landscape and drainage proposals to be consistent in SuDS proposals.

1. SURFACE WATER DESIGN

The Site Drainage Layout Drawing No.5845/01 shows the proposed surface water layout. Permeable paving is proposed for the parking zone that acts as an attenuation/ infiltration zone. All surface run-off is collected and discharges into the permeable paving sump. An overflow outlet is then connected to an inspection chamber which then discharges to the existing public surface water.

The surface water design methodology is in accordance with the criteria below:

- The pipe network is designed for a rainfall intensity of 50mm/hr, BS8301 8.8.2 or 1 in 2 year return period;
- Allowance for 20% Climate change;
- Attenuation storage in accordance with SUDS & South Dublin City Council requirements;
- Design for interception of the first 5mm of all rainfall events;

Designed based on Wallingford method outlined in the CIRIA Report R156 (1996) and SuDS Manual C753

1.1. EXISTING SITE DATA

Average annual rainfall data obtained from Met Eireann for the area is shown in the figure below.

Met Eireann Return Period Rainfall Depths for sliding Durations Irish Grid: Easting: 303878, Northing: 235466,

Interval			1	Years														
DURAT	ION 6m	onths,	lyear,	2,	3,	4,	5,	10,				75,			200,			
5 m	ins	2.4,	3.5,	4.1,	5.0,	5.7,							16.3,					
10 m	ins	3.3,	4.8,	5.7,	7.0,								22.7,					
15 m	ins	3.9,	5.7,	6.7,	8.3,								26.7,					
30 m	ins	5.1,	7.4,	8.7,	10.6,	12.0,	13.0,	16.5,	20.5,	23.1,	26.9,	30.3,	33.0,	37.2,	40.4,	43.1,	N/A,	
1 hor	urs	6.8,	9.7,	11.3,		15.4,											N/A ,	
2 hor	urs	8.9,	12.6,	14.6,		19.7,												
3 hor	urs	10.5,	14.8,	17.0,	20.5,	22.8,	24.6,	30.4,	37.0,	41.4,	47.5,	52.9,	57.1,	63.6,	68.7,	72.8,	N/A ,	
4 hor	urs	11.8,	16.5,	19.0,	22.7,	25.2,	27.2,	33.5,	40.7,	45.4,	52.0,	57.9,	62.4.	69.3,	74.7,	79.2,	N/A,	
6 ho	urs	13.9.	19.3,	22.1,	26.3,	29.2,	31.4,	38.5,	46.5,	51.8,	59.1,	65.6,	70.6,	78.3,	84.3,	89.2,	N/A ,	
9 hor	urs	16.4,	22.5,	25.7,		33.7,												
12 hor	urs	18.4,	25.1,	28.7,	33.9,	37.4,	40.1,	48.8,	58.5,	64.8,	73.6,	81.4,	87.3,	96.4,	103.4,	109.2,	N/A ,	
18 ho	urs	21.6.	29.3,	33.4.	39.3,	43.3,	46.3,	56.1,	66.9,	74.0,	83.7,	92.3,	98.9,	108.9,	116.6,	122.9,	N/A ,	
24 hor	urs		32.8,	37.2,	43.7,	48.0,	51.3,	61.9,	73.6,	81.2,	91.7,	100.9,	108.0,	118.7,	126.9,	133.7,	157.1,	
2 d	avs	30.4,		44.9.	52.0,	56.7,	60.2,	71.5,	83.8,	91.7,	102.5,	111.9,	119.0,	129.8,	138.0,	144.8,	167.8,	
3 d			45.8,	51.1.	58.7.	63.7,	67.5,	79.4,	92.3,	100.4,	111.6,	121.2,	128.5,	139.5,	147.8,	154.6,	177.8,	
4 d		39.7,				69.8,												
6 d			59.7,	65.9.	74.8.	80.4,	84.7,	98.1,	112.4,	121.3,	133.4,	143.7,	151.5,	163.1,	171.9,	179.0,	203.0,	
8 d			67.6,	74.2,	83.7,	89.7,	94.3,	108.5,	123.5,	132.8,	145.4,	156.1,	164.2,	176.2,	185.3,	192.6,	217.1,	
10 d	avs		74.7,	81.8.	91.8,	98.2,	103.0,	117.9,	133.5,	143.2,	156.3,	167.4,	175.7,	188.1,	197.3,	204.8,	230.0,	
12 d			81.3,	88.8.	99.3.	106.0,	111.0,	126.5,	142.7,	152.8,	166.3,	177.7,	186.3,	199.0,	208.5,	216.1,	241.8,	
16 d			93.6.	101.7.	113.1,	120.3,	125.7,	142.4,	159.6,	170.3,	184.5,	196.5,	205.5,	218.8,	228.7,	236.7,	263.2,	
20 d			104.9,			133.4,												
25 d			118.0,	127.4,	140.4,	148.6,	154.8,	173.4,	192.6,	204.4,	220.0,	233.1,	242.8,	257.1,	267.8,	276.4,	304.7,	
NOTES																		

NOTES:
N/A Data not available
These values are derived from a Depth Duration Frequency (DDF) Model
For details refer to:
'fitzgerald D. L. (2007), Estimates of Point Rainfall Frequencies, Technical Note No. 61, Met Eireann, Dublin',
Available for download at www.met.ie/climate/dataproducts/Estimation-of-Point-Rainfall-Frequencies_TN61.pdf

Qbar was calculated in accordance with the Wallingford Method for the existing site:

Site Area = 360m2

Qbar = 0.075 I/s

			1. SITE DETAILS						
Site Area (m2):	360	m2							
Public open space 0		m2	Not draining to system						
Site Area (HA): 0.036		HA	Site area minus POS						
SAAR (mm): 784		mm	Source: www.met.ie/climate/services						
Soil Type: 2		-	Reference: Flood Studies Report (NERC, 1975)						
SPR:	0.3	-	Reference: Flood Studies Report (NERC,1975)						
		2. II	H124 METHOD (WALLINGFORD)						
s Qbar < 50 HA?	Yes	Use Me	ethod 1 Below						
1. QBAR BASED OF	N AREA RATI	O (AREA	A<50 HA)						
Area Ratio:	0.0007								
Qbar (50 HA):	0.1040	m3/s	Calculation: Qbar=0.00108*((0.01*Site Area)^0.89)*(SAAR^1.17)*SPR^2.1						
Qbar (Actual):	0.0001	m3/s	Calculation: Qbar(50HA)*Area Ratio						
Qbar (Actual):	0.0749	l/s	Calculation: Qbar Actual (m3/s)*1000						

1.2. ATTENUATION / INFILTRATION DESIGN

The storage volume is determined in accordance with Wallingford method based on correlations between storage requirements and hydrological and hydraulic characteristics of the site (ww.uksuds.com).

The calculated volume based on the IH124 method that follows is 8.9m3 at the minimum 2 l/s acceptance rate into the Public Sewer.

1.3. PERMEABLE PAVING

Porous paving is proposed for the 117m2. A percolation test is recommended however the attenuation sump has been sized based on attenuation and neglecting infiltration losses.

All surface run-off from the roof zones will enter the sump in the coarse graded aggregate layer through a perforated pipe. A perforated outlet pipe is provided should overflow conditions occur which discharges to Public Surface water network.

Vehicle access is required including emergency vehicles, therefore load category B was chosen. The proposed build-up is described below:

- 1. TOBERMORE HYDROPAVE 200x100x80mm
- 2. 50mm Thickness of 6.3-2mm grit to BS EN13242:2002
- 3. 350MM Thickness 4/20mm coarse graded aggregate to BS en 13242:2002
- 4. Impermeable to BS 7533 Part 13

The total stone volume provided is 41m3, which at 40% porosity corresponds to 16.4 m3 of available storage.

Green roof SuDS

Green roof can provide benefits in terms of reducing peak flow rates.

The Benefits of Installing Green Roof

Reducing the amount of surface water running off the Roof and so reducing the risk of flooding. Completed projects show a reduced annual run-off of at least 40% and more usually 60-70%. In some cases, for Intensive Green Roofs, the water retention can be up to 90%.

Providing habitat (homes), shelter and feeding opportunities for wildlife.

Contribute to sustainable drainage systems and water quality improvement.

Helping biodiversity

Improving the character and appearance of the building and the wider area

Providing extra heat and noise insulation.

Keeping the building cool in the summer.

Increasing the lifespan of the Roof membrane.

Helping to reduce the amount of dust and pollutants in the air.

Creating new open space for relaxation, providing potential for the creation of usable green spaces.

Site Area	(m2)·	360	m2							
			1112							
Public Op	and the second section in the second section	0	LIA							
Site Area (HA): 0.036 SAAR (mm): 784			HA mm		Courses	vw.met.ie/clin				
Soil Type:		2	mm					1075)		
SPR:		0.3	-		The second second	Flood Studies		The second secon		
SFK.		0.5			kejerence.	: Flood Studies	Keport (NEKC	,1975)		
				2. RUN-O	FF AREAS	L				
					Area	Coeffic	cient of			
Si	urrace Rui	n-off zones	(m2)		(m2)	Perme	ability	Effectiv	ve Area (m2)	
	Imperm	eable Area			360		1		360.00	
La	ndscaping	and or gree	n area		0	0	.8	0.00		
	Partially p	emeable a	rea		0 0.3			0.00		
							Total Area	360.00		
	Use Oh	par from Me	thod 1 (Areas	50 HA) or Meth	nod 2 (Area	>50 HA)?		M	ethod 1	
	000 00	J OITI WO	I priod	20 i) of Midtl	.50 2 (/100		Qbar	0.07	I/s	
						Minimu	m flow rate	2.00	I/s	
					In	terception ra		5.00	mm	
						mate Change		20	%	
				30 YEAR RET						
Tim s	Mina	0	Max Rainfall for 30 Year		Area	El (Va)	Volume	QBAR	Attenuation	
Time 5 min	Mins 5	Secs	Storm (m)	rainfall (m)	(m2)	Flow (I/s)		vol.(m3)	required (m3	
10 min		300	0.0112	0.01344	360	10.13	3.04	0.60	2.44	
	10	600	0.0156	0.01872	360	8.23	4.94	1.20	3.74	
15 min	15	900	0.0184	0.02208	360	6.83	6.15	1.80	4.35	
30 min	30 60	1800	0.023	0.0276	360	4.52	8.14	3.60	4.54	
60 min 2 hour	120	3600 7200	0.0288	0.03456	360	2.96	10.64	7.20	3.44	
3 hour	180	10800	0.0361 0.0412	0.04332	360 360	1.92	13.80	14.40	-0.60	
4 hour	240	14400	0.0412	0.04944 0.05424	360	1.46	16.00	21.60	-5.60	
6 hour	360	21600	0.0452	0.05424	360	0.95	17.73	28.80	-11.07	
9 hour	540	32400	0.0518	0.00192	360	0.93	20.49	43.20	-22.71 -41.20	
12 hour	720	43200	0.0566	0.07050	360	0.60	26.11	64.80		
18 hour	1080	64800	0.0040	0.07752	360	0.46	30.04	86.40 129.60	-60.29 -99.56	
24 hour	1440	86400	0.0809	0.00044	360	0.48		172.80	F 45 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	
48 hour	2880	172800	0.0009	0.09706	360	0.30	33.15 37.94		-139.65 -307.66	
40 HOUI	2000	172000	0.092	100 YEAR RE	-	- Contract of the last of the	31.94	345.60	-307.00	
Time	Mins	Soon	Max Rainfall for 100 Year Storm (m)	Climate change Factored	Area		Volume	QBAR	Attenuation volume	
5 min	5	Secs 300	0.0161	0.01932	(m2) 360	17.18	(m3) 5.16	vol.(m3) 0.60	required (m3 4.56	
10 min	10	600	0.0224	0.02688	360	13.13	7.88	1.20	6.68	
15 min	15	900	0.0264	0.03168	360	10.67	9.60	1.80	7.80	
30 min	30	1800	0.033	0.0396	360	6.92	12.46	3.60	8.86	
60 min	60	3600	0.0403	0.04836	360	4.34	15.61	7.20	8.41	
2 hour	120	7200	0.0498	0.05976	360	2.74	19.71	14.40	5.31	
	180	10800	0.0564	0.06768	360	2.09	22.56	21.60	0.96	
3 hour	240	14400	0.0616	0.07392	360	1.72		28.80	-3.99	
	200	21600	0.0697	0.08364	360	1.31		43.20	-14.89	
4 hour	360				360	1.00		64.80	-32.52	
4 hour 6 hour	540	32400	0.0789	0.09468	300					
4 hour 6 hour 9 hour			0.0789 0.0862	0.09468	360	0.82	35.44	86.40	-50.96	
4 hour 6 hour 9 hour 12 hour	540	32400							-50.96 -89.24	
3 hour 4 hour 6 hour 9 hour 12 hour 18 hour 24 hour	540 720	32400 43200	0.0862	0.10344	360	0.82	35.44	86.40		
4 hour 6 hour 9 hour 12 hour 18 hour	540 720 1080	32400 43200 64800	0.0862 0.0976	0.10344 0.11712	360 360	0.82 0.62	35.44 40.36	86.40 129.60	-89.24	