

Coughlan Consulting Engineering

Consulting Structural & Civil Engineering
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22035

5th July 2022

SD22B/0124 - Condition No.2 for 54 Beechfield Road, Dublin 12

2. Drainage - Surface Water.

Prior to the commencement of development, the applicant/developer shall submit the following for the written agreement of the Planning Authority:

- (1) a report showing site specific soil percolation test results and design calculations for the proposed soakaway in accordance with BRE Digest 365 Soakaway Design.
- (2) a revised drawing showing plan and cross-sectional views, dimensions, and location of proposed soakaway. Any proposed soakaway shall be located fully within the curtilage of the property and shall be:
- (i) At least 5m from any building, public sewer, road boundary or structure.
- (ii) Generally, not within 3m of the boundary of the adjoining property.
- (iii) Not in such a position that the ground below foundations is likely to be adversely affected.
- (iv) 10m from any sewage treatment percolation area and from any watercourse / floodplain.
- (v) Soakaways must include an overflow connection to the surface water drainage network.
- (3) Should a soakaway prove not to be feasible, then the applicant shall submit the following:
- (a) Soil percolation test results demonstrating a soakaway is not feasible
- (b) A revised surface water layout drainage drawing for the development showing the inclusion of alternative SuDS (Sustainable Drainage Systems) features such as rain gardens and rain planter boxes

Response:

Coughlan Consulting Engineering were appointed by Cillian and Gayle McCormack Doyle of 54 Beechfield Road, Dublin 12 to prepare an engineering response to Item 2

Coughlan Consulting Engineering instructed an infiltration test on site to determine the required size for the soakaway test. Based on these tests, Coughlan Consulting Engineering can confirm that a Soakaway is suitable to be used on this site. The soakaway will be minimum 5m from the proposed extension and will be 2.6m clear of the side boundary wall. This is slightly under the suggested 3m and is hoped is considered acceptable.

Please see CCE-SK01 and SK02 attached in Appendix A and Soakaway calculations can be found in Appendix B.

Robert Coughlan.

For and on behalf of: Coughlan Consulting Engineering

25 Kiltipper Avenue,

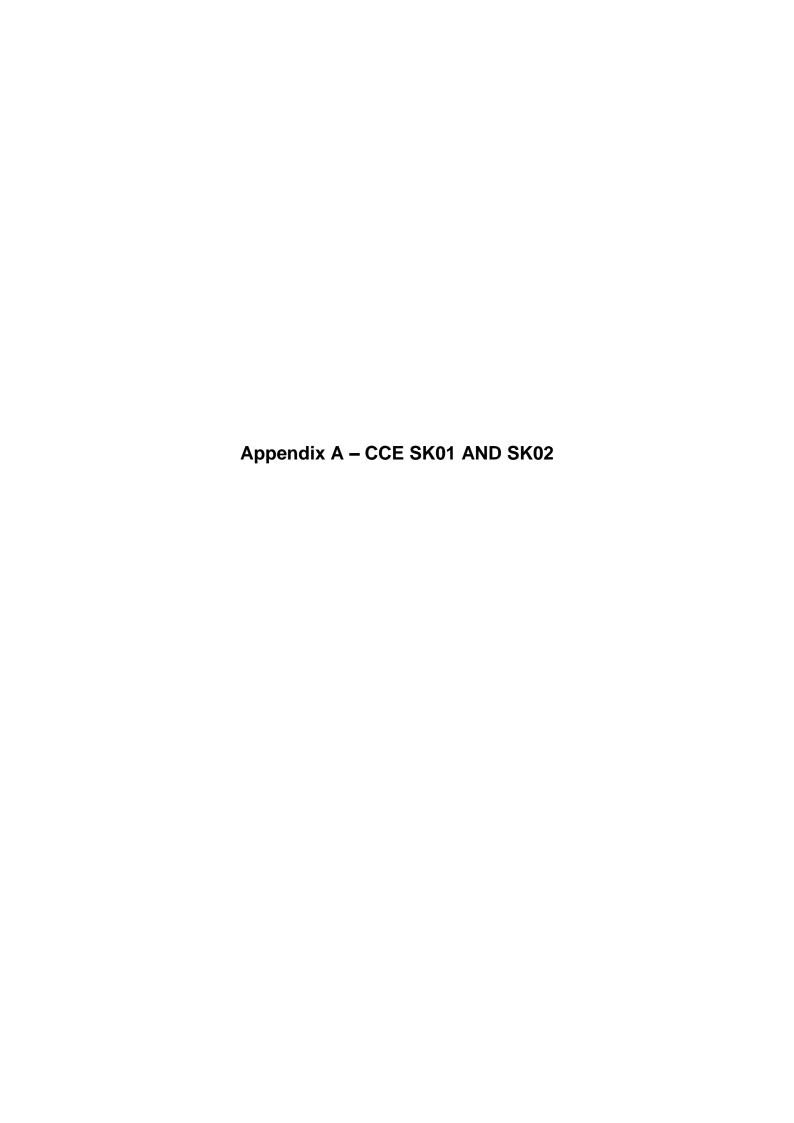
Aylesbury, Tallaght, Dublin 24.

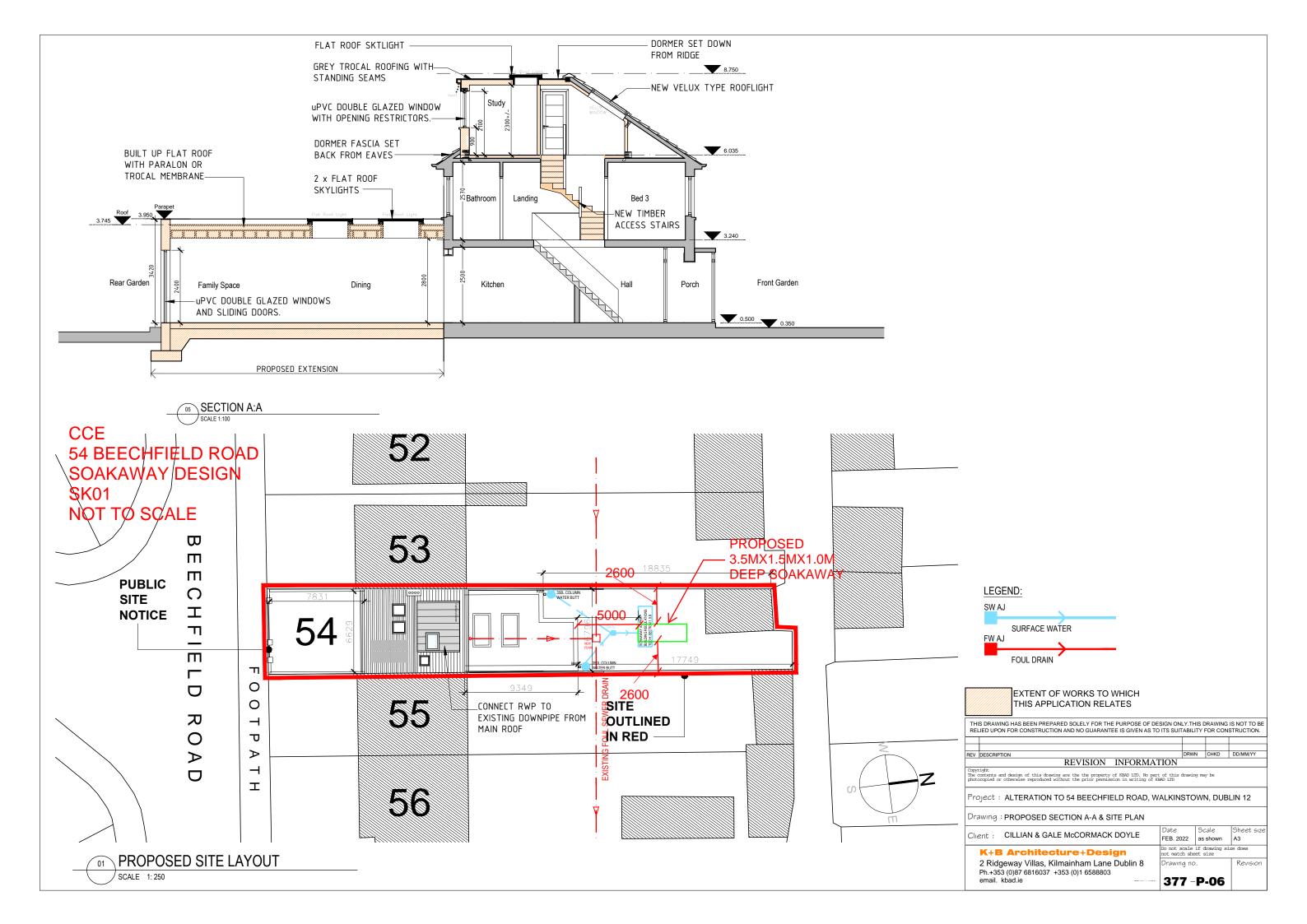
Signed:

Name: Robert Coughlan

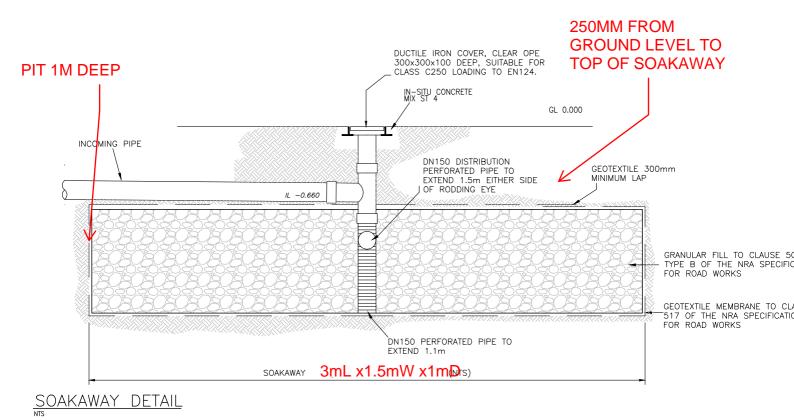
Qualifications: BE, CEng, MIEI, MIStuctE, BER Assessor.

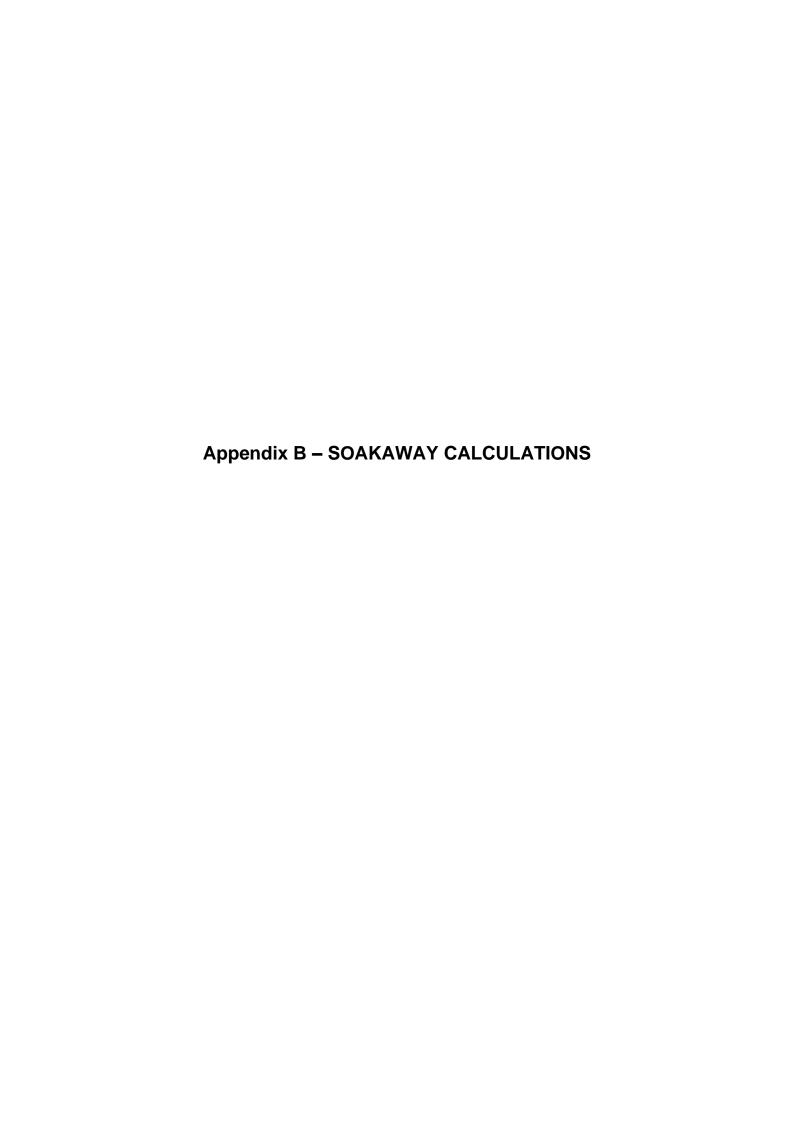
Professional Body/Membership No. MIEI No.: 053927





CCE 54 BEECHFIELD ROAD SOAKAWAY DESIGN SK02 NOT TO SCALE





Tekla.Tedds Coughlan Consulting Engineering	Project 54 Beechfield Road				Job no. 22035	
25 Kiltipper Avenue Aylesbury Tallaght, D24	Calcs for	Soakaway Desisgn			Start page no./Revision	
	Calcs by RC	Calcs date 05/07/2022	Checked by	Checked date	Approved by RC	Approved date

SOAKAWAY DESIGN

In accordance with BRE Digest 365 - Soakaway design

Tedds calculation version 2.0.04

Design rainfall intensity

5-year return period rainfall of 60 minutes duration M5_60min = **19.0** mm

Increase of rainfall intensity due to global warming pclimate = 0 %

Soakaway / infiltration trench details

Soakaway type Rectangular Minimum depth of pit (below incoming invert) d = 858 mm Width of pit w = 1500 mm Length of pit l = 3500 mm Percentage free volume $V_{\text{free}} = 30 \text{ }\%$

Soil infiltration rate (BRE digest 365)

75% depth of pit $d_{75} = (d_{trial} \times 0.75) = \textbf{750.00} \text{ mm}$ 50% depth of pit $d_{50} = (d_{trial} \times 0.50) = \textbf{500.00} \text{ mm}$ 25% depth of pit $d_{25} = (d_{trial} \times 0.25) = \textbf{250.00} \text{ mm}$

Test 1 - time to fall from 75% depth to 25% depth T1 = 200 minTest 2 - time to fall from 75% depth to 25% depth T2 = 200 minTest 3 - time to fall from 75% depth to 25% depth T3 = 200 min

Longest time to fall from 75% depth to 25% depth $t_{lg} = max(T1, T2, T3) = 200 min$

Storage volume from 75% to 25% depth $V_{p75_25} = \left(I_{trial} \times b_{trial} \times (d_{75} - d_{25})\right) \times V_{trial} = \textbf{0.50} \text{ m}^3$ Internal surface area to 50% depth $a_{p50} = \left(\left(I_{trial} \times b_{trial}\right) + \left(I_{trial} + b_{trial}\right) \times 2 \times d_{50}\right) = \textbf{3.00} \text{ m}^2$

Surface area of soakaway to 50% storage depth $A_{s50} = 2 \times (I_{trial} + b_{trial}) \times d_{trial} / 2 = 2.000 \text{ m}^2$ Soil infiltration rate $f = V_{p75_25} / (a_{p50} \times t_{lg}) = 13.9 \times 10^{-6} \text{ m/s}$ Wetted area of pit 50% full $a_{s50} = I \times d + w \times d = 4292415 \text{ mm}^2$

Table equations

Inflow (cl.3.3.1) $I = M30 \times A$ Outflow (cl.3.3.2) $O = a_{s50} \times f \times D$ Storage (cl.3.3.3) S = I - O

Duration, D (min)	Growth factor Z1	M5 rainfalls (mm)	Growth factor Z2	30 year rainfall, M30 (mm)	Inflow (m³)	Outflow (m³)	Storage required (m³)
5	0.36;	6.8;	1.45;	9.9;	0.50;	0.02;	0.48
10	0.51;	9.7;	1.49;	14.4;	0.72;	0.04;	0.68
15	0.62;	11.8;	1.50;	17.7;	0.89;	0.05;	0.83
30	0.79;	15.0;	1.53;	22.9;	1.15;	0.11;	1.04

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Aylesbury Tallaght, D24		Soakawa	y Desisgn		:	2	
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Duration, D (min)	Growth factor Z1	M5 rainfalls (mm)	Growth factor Z2	30 year rainfall, M30 (mm)	Inflow (m³)	Outflow (m³)	Storage required (m³)
60	1.00;	19.0;	1.54;	29.3;	1.46;	0.21;	1.25
120	1.22;	23.2;	1.54;	35.6;	1.78;	0.43;	1.35
240	1.48;	28.1;	1.52;	42.8;	2.14;	0.86;	1.28
360	1.67;	31.7;	1.51;	47.8;	2.39;	1.29;	1.10
600	1.90;	36.1;	1.48;	53.6;	2.68;	2.15;	0.53
1440	2.42;	46.0;	1.44;	66.2;	3.31;	5.15;	0.00

Required storage volume

Soakaway storage volume

 $S_{req} = 1.35 \text{ m}^3$

 $S_{act} = I \times d \times w \times V_{free} = \textbf{1.35} \ m^3$

PASS - Soakaway storage volume

Time for emptying soakaway to half volume

 t_{s50} = $S_{req} \times 0.5$ / $(a_{s50} \times f)~$ = 3hr 8min 43s

PASS - Soakaway discharge time less than or equal to 24 hours