

PROPOSED RESIDENTIAL DEVELOPMENT AT GORDON PARK, OLD NAAS ROAD, DUBLIN 22

Surface Water Management Plan



April 2022

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Document Verification

Client: Greenwalk Developments Limited

Job Title: Proposed Residential Development at Gordon Park

Job No: 21003

Document Ref: Surface Water Management Plan (SWMP)

File Name: 21003-TJOC-ZZ-ZZ-RP-C-3702.doc

Revision Number	Suitability	Description of Revision	Prepared By	Checked By	Approved By	Date of Issue
C01	AP	Issued for Planning/Additional Information	JM	NS	JM	21.04.2022

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1. INTRODUCTION AND SITE DESCRIPTION

Greenwalk Developments Ltd. are seeking to develop 77 No. residential units at a site on the Old Naas Road which is currently occupied by Clondalkin Rugby Football Club (RFC). Clondalkin RFC have recently acquired other lands in the area to allow their club and playing facilities to expand to meet their growing requirements.

The site, named locally as Gordon Park, is bounded by the Roadstone Group Sports Club and sports facilities to the north and east, the Silken Park Residential Development to the south and the Old Naas Road to the west. The site is relatively flat, with the ground levels falling from a level of 95.20mOD at the southwest corner of the site to a level of 92.00mOD at the existing site entrance at the northwest corner of the site.

The Fettercairn Stream, a tributary of the Camac River, abuts the eastern boundary of the site and runs northwards for a distance of approximately 80m from the south eastern corner of the site boundary, before turning 90 degrees and heading in an easterly direction away from the site. Figure 1 below shows the location of the site.

Set out below is relevant information relating to the proposed surface water management plan (SWMP) for the proposed development. This SWMP should be read in conjunction with the engineering planning drawings, landscape drawings and all other planning submission documents/reports.

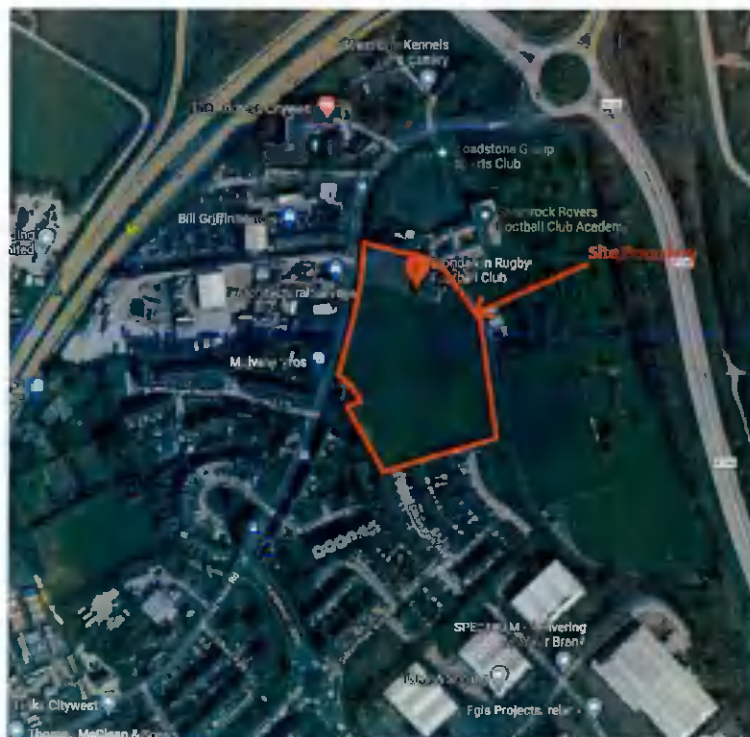


Figure 1: Site Location

2. EXISTING SURFACE WATER INFRASTRUCTURE

Existing service record drawings for the area in proximity to the site were obtained from the Local Authority and Irish Water. A Ground Penetrating Radar survey was also undertaken on the Old Naas Road in proximity to the site to establish the presence of existing services that may not be recorded on the Local Authorities/Utility Providers service record drawings. Following a review of all available information, it was determined that there is no public surface water drainage infrastructure in the Old Naas Road in proximity to the site. However, as noted above, the Fettercairn Stream abuts part of the eastern boundary of the site as shown in Figure 2 below.

A detailed topographical survey of the site was undertaken, and on review, it is evident that the existing watercourse on the east boundary of the site is not located at the lowest part of the site. Therefore, whilst it was clear that the majority of the site (approximately 85 % of the site) can discharge by gravity to the existing watercourse, a number of options for draining the northwest corner of the site would need to be considered as part of the overall surface water management plan for the proposed development. Figure 2 below provides an overview of the site levels and shows the extent of site (15% of the site) at the northwest corner of the site that cannot be drained to the existing watercourse based by gravity due to the existing ground levels.

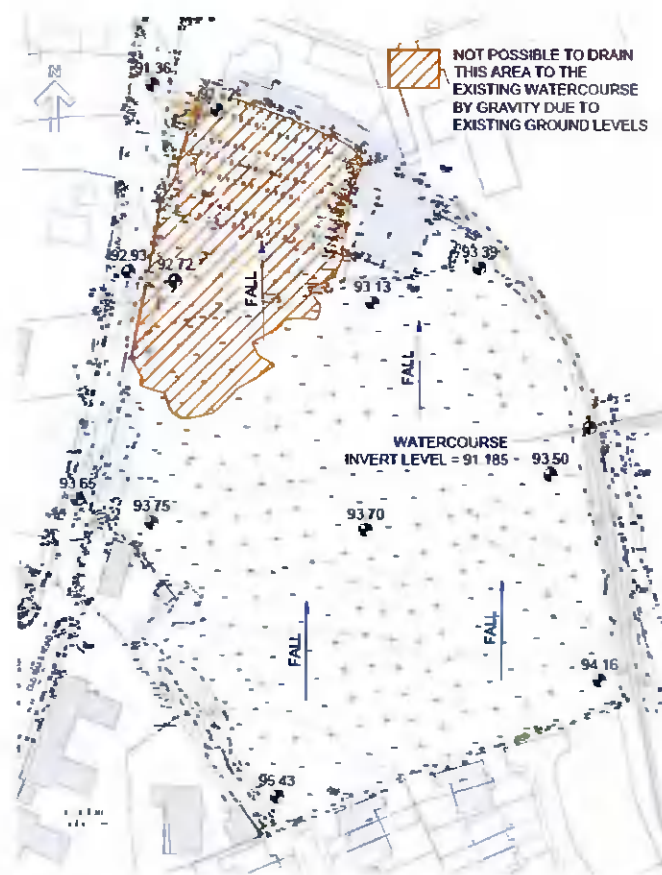


Figure 2: Overview of Site Levels

3. PROPOSED SURFACE WATER DRAINAGE & SUDS MEASURES

3.1 Existing Ground Conditions

A number of infiltration tests were carried out on the site, refer to Drg No. 21003-TJOC-ZZ-ZZ-DR-C-0054 for test locations and Appendix A for test results. The results indicate that, due to the existing clay soil conditions, infiltration on the western side of the site is very poor. The infiltration rate on the eastern side of the site is slightly better, but the underlying soil conditions are clay which is not a reliable media for long term infiltration. The infiltration test results obtained for this site are consistent with the results of infiltration tests carried out in similar ground conditions in the Citywest region. The clay soils (Brown and Black Boulder Clays) that are evident in the Citywest region do not lend themselves to reliable long-term infiltration of surface water.

Based on the investigations, it is proposed to adopt SuDS measures and SuDS features that will allow surface water to infiltrate to ground if local infiltration is available. However, based on the existing ground conditions, it is not possible to design the surface water system to discharge all surface water to the ground via infiltration. Therefore, whilst the SuDS features will provide an opportunity to infiltrate surface water into the ground, the SuDS features will be designed with outlet and overflow features to allow the surface water generated in peak storm events to discharge to the surface water site network where it will be stored and released from the site at the pre-development greenfield run-off rate.

3.2 Greenfield Run-off Rate

The pre-development greenfield run-off rate for the site has been determined using the HR Wallingford Greenfield run-off rate calculator. A copy of the calculation is provided in Appendix B. The site parameters used to determine the greenfield run-off rate are set out below.

- Site area of 2.282 hectares,
- Soil type 2
- SAAR of 873mm.
- Pre-development Greenfield Run-off Rate = 5.4 l/sec

A flow control device with a surface water discharge rate of 5.4 l/sec is shown on Drg No. 21003-TJOC-ZZ-ZZ-DR-C-0065. The flow control device is located at the surface water discharge point from the site and therefore limits the discharge rate from the site to the pre-development greenfield run-off rate. We refer to the extract below which shows the location of the flow control device that limits the surface water discharge from the site to 5.4 l/sec.

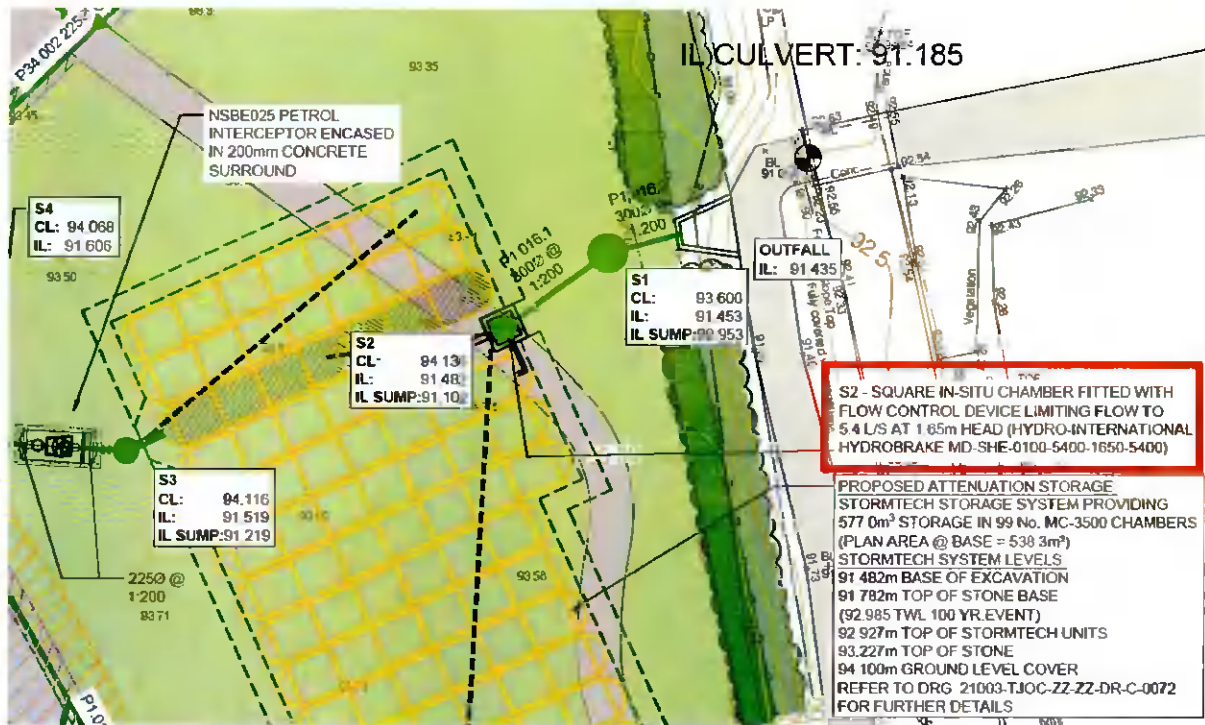


Figure 3 – Location of Surface Water Flow Control Device

We note that a second flow control device within the site surface water network is also indicated on Drg No 21003-TJOC-ZZ-ZZ-DR-C-0065. The purpose of the second flow control device is to hold back and control surface water flow in the surface water network within the site. The limit set for the second flow control device is 6.5l/sec. However, there is only one surface water discharge point from the site and the flow control device at this point is set at 5.4l/sec.

It is noted that Soil type 2 has been used to determine the greenfield run-off rate for the site. Given the lack of infiltration that is available on the site, the use of Soil type 2 is conservative and a Soil type 3 or even 4 could be used to determine the pre-development greenfield run-off rate. However, for this project, it is proposed to adopt a Soil type 2 to determine the greenfield run-off rate and to limit the surface water discharge from the site to 5.4l/sec.

3.3 Proposed SuDS Measures

As noted in Section 3.1 above, it is not possible to discharge all surface water to the ground via infiltration. Therefore, a hybrid solution of infiltration to the ground where possible and collection of surface water with a piped network is proposed for this development. Based on the results of the site infiltration tests, the proposed surface water drainage and associated SuDS features have been designed with an appropriate allowance for infiltration to suit the existing clay ground conditions

The typical surface water arrangement for the proposed houses is as follows. The driveways of houses will be provided with permeable paving. Rainwater from the front roofs of the houses and rainfall on the driveways will discharge to the subbase of the permeable paved driveways. The base of the driveway build-up will be provided with a geotextile membrane to allow surface water to infiltrate to ground where existing ground conditions permit. Where infiltration is not possible, surface water will be stored within the permeable subbase and released to the surface water network via a fin drain within the subbase. For surface water calculation purposes, the infiltration rate adopted for the base of the driveway subbase is 2.7×10^{-6} m/sec or 0.010m/hr. This infiltration rate is lower than the infiltration rate obtained for the soakaway tests undertaken on the east side of the site, but is consistent with an infiltration rate for clay soils with low/poor infiltration as encountered in the soakaway pits on the west side of the site.

Rainwater from the rear roofs of the houses will discharge into linear infiltration trenches located in the rear gardens of houses. Similar to the permeable paving, the linear infiltration trenches will be provided with a geotextile membrane around its perimeter to allow surface water to infiltrate to ground where existing ground conditions permit. A perforated drain/pipe will be provided within the infiltration trench which will be connected to the surface water network. This "overflow" arrangement will cater for larger rainfall events which cannot infiltrate into the ground.

As for the driveways, for design purposes, only a nominal allowance/rate for infiltration has been adopted in the surface water calculations for the infiltration rate into the ground. This is consistent with the design of infiltration features in clay soil conditions. However, in practice, the provision of SuDS features such as permeable paving and filter/infiltration drains may allow for greater levels of infiltration than the nominal rates adopted in the calculations.

Given that it is proposed that the roads and footpaths will be taken in charge in the future, they have been designed and detailed in accordance with the Local Authority taking in charge standards. For the site roads and footpaths, surface water run-off will be collected in road gullies or diverted to dropped kerbs and subsequently discharged into a number of SuDS features such as tree pits, bioretention areas and filter strips. The SuDS features will be provided with subsurface storage media with a minimum voids ratio of 30% to generate surface water storage within the SuDS feature.

These SuDS features for the roads and footpaths will be provided with outlet pipes/overflow pipes to allow surplus surface water that cannot infiltrate to ground to discharge to a surface water piped network within the roads. The overall balance of surface water storage/attenuation for the site will be provided in two main areas at the site, namely at the northwest of the site and at the east of the site adjacent to the proposed surface water outfall location as shown on the surface water drainage drawings.

As set out above, the proposed development incorporates a 2-Stage treatment approach for the surface water design. Full details of the proposed surface water design and details are included on the drawings and calculations submitted with the Planning Application. A summary of the proposed Surface Water Management Train for the proposed development is provided in Figures 3 & 4 below.

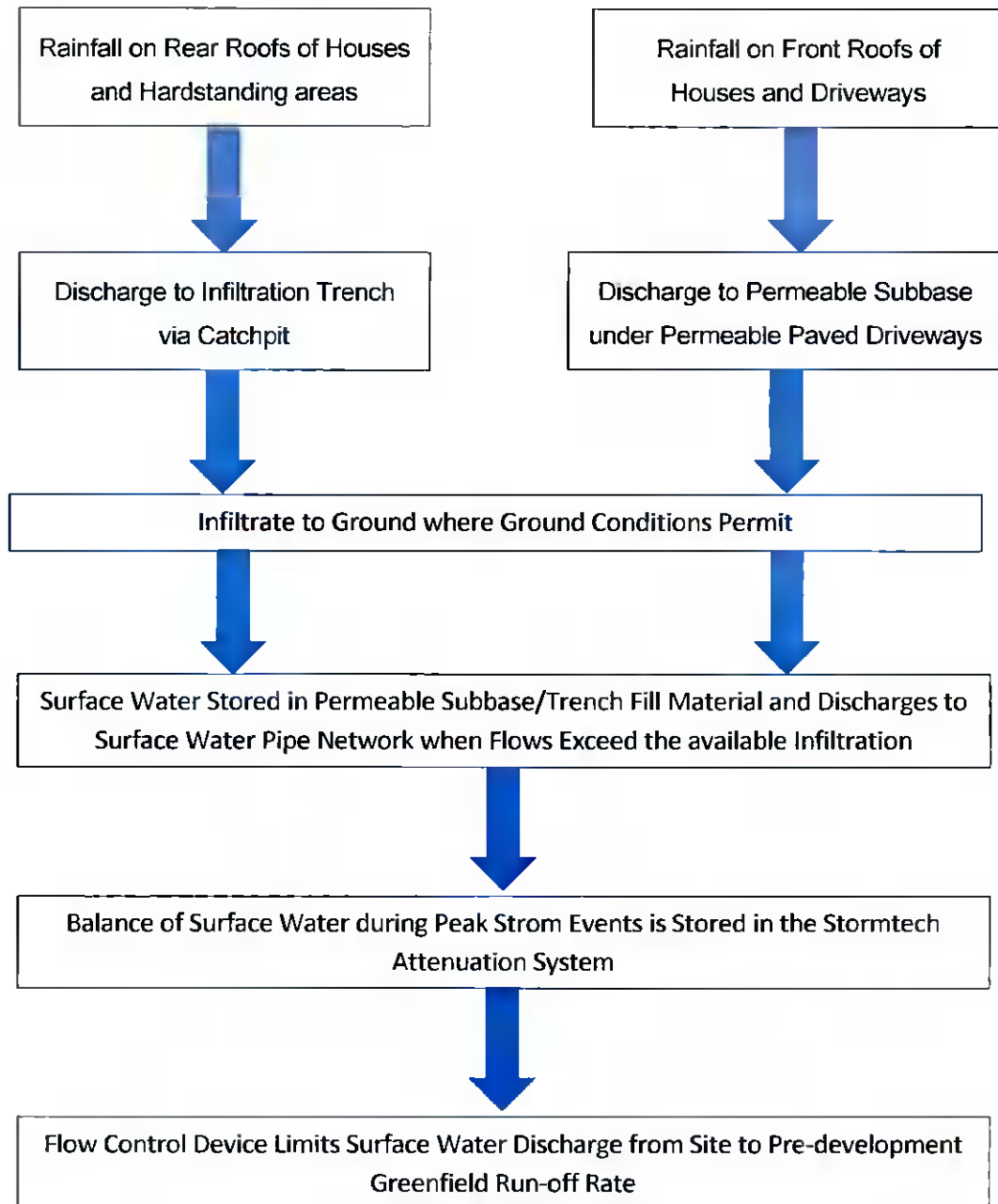


Figure 3: Surface Water Management Train for House Units

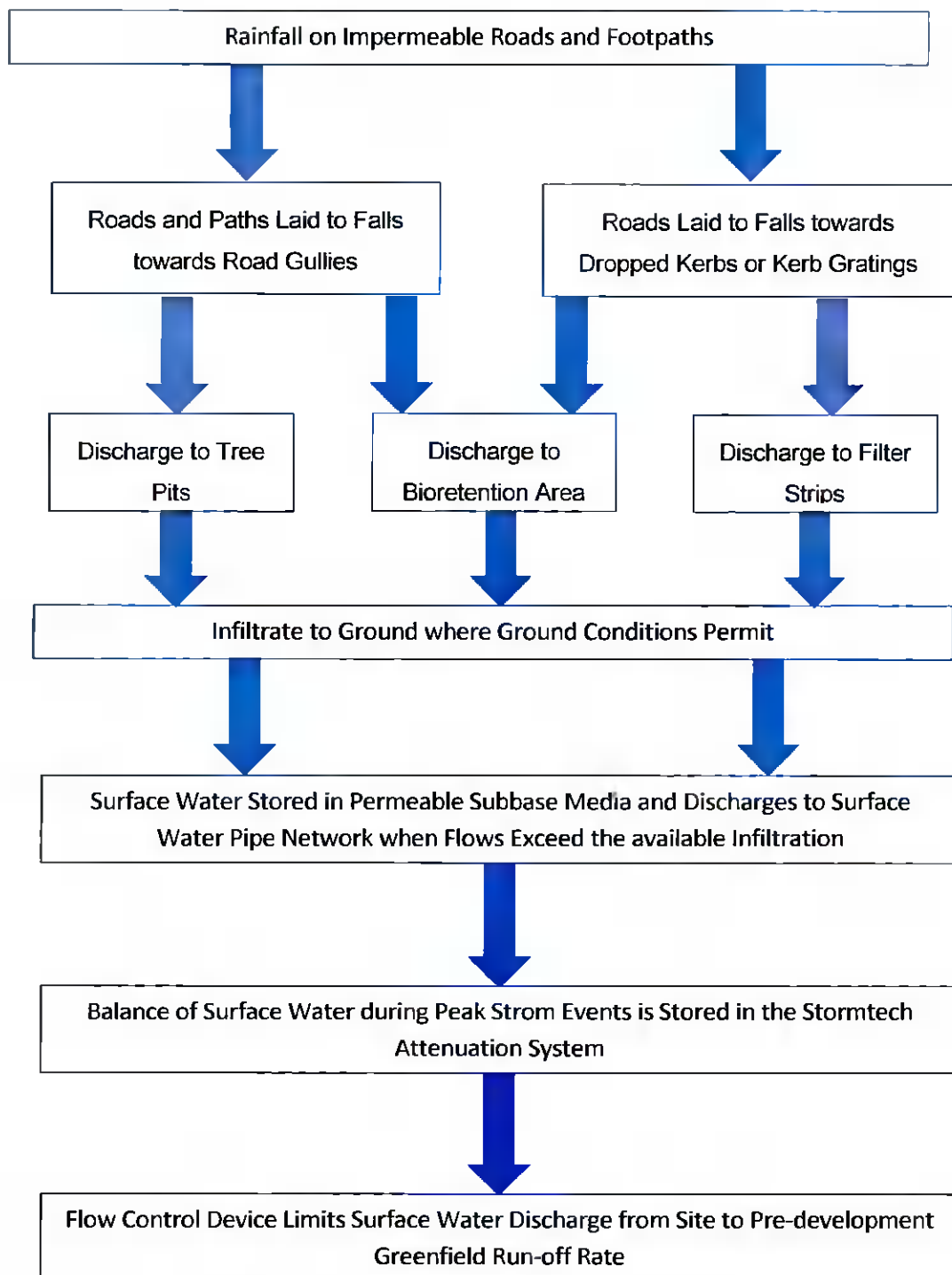


Figure 4: Surface Water Management Train for Site Roads and Footpaths

To create sufficient falls in the surface water piped network and allow it to discharge to the existing watercourse on the east side of the site, it is proposed to locally raise the ground levels on the northwest corner of the site. The levels will be raised by approximately 500mm to 1200mm locally, but the design of the scheme has been developed to avoid the need for extensive retaining structures to accommodate this modest increase in ground level. The sides of the raised areas will be sloped down to existing ground levels along the northwest side of the site and landscaped to tie into the existing tree lined boundary of the site.

3.4 Surface Water Design/Calculations

The surface water drainage network for the proposed development has been designed using the Innovyze MicroDrainage design software which is based on the Wallingford procedure. The surface water drainage pipe network has been sized based on 1 in 2-year return event.

The proposed surface water drainage system has been designed not to flood in a 1 in 30-year event and flooding occurring during a 1 in 100-year event will be retained on site in accordance with the Greater Dublin Strategic Drainage Study Design Criteria. The existing site is 2.282hA in size. For this planning application the proposed site area is 2.282hA i.e. the full area of the site. The proposed surface types and associated areas are shown on Drg. No. 21003-TJOC-ZZ-ZZ-DR-C-0067 which accompanies this application.

The total impervious area of the proposed development including permeable paved areas that are connected to surface water piped network is 1.29hA. The total permeable Public Open Space, Private Open Space and any footpaths that drain onto permeable areas is 0.992hA.

As noted above, the surface water design has been carried out using Microdrainage software which uses the Modified Rational Method for surface water design. When using the modified rational method of design, it is recommended to apply a run-off factor of 100% to all impermeable areas and a run-off factor of 0% to all permeable areas (such as private open spaces and public open spaces). This is consistent with the Surface Water Design Criteria in Table 6.4 of Vol 2 of the GDSDS (see extract is figure 5 overleaf) and with Section 24.11.2 of the SUDS Manual, CIRIA C753.

Parameter	Surface Water Sewers
Minimum depth	1.2m cover under highways 0.9m elsewhere
Maximum depth	Normally 5m
Minimum sewer size	225mm
Runoff factors for pipe sizing	100% paved and roof surfaces 0% off pervious surfaces
Rainfall for initial pipe sizing	50mm/hr rainfall intensity
Minimum velocity (pipe full)	1.0m/s
Flooding	Checks made for adequate protection * No flooding for return period less than 30 years except where explicitly planned Simulation modelling is required for sites greater than 24ha**
Roughness – ks	0.6mm

Table 6.4 Surface Water Design Criteria

Figure 5: Table 6.4 of GSDSDS Vol. 2

A summary of the contributing areas to the surface water design and surface types is provided in Table 1 overleaf. The limiting flow discharge from the proposed development is set at the pre-development greenfield run-off rate of 5.4 l/s as set out in Section 3.2 above. Rainfall data has been obtained from Met-Eireann for the site and the M5-60 figure and ratio of M5-60/M5-2day are used in the Microdrainage design software for this project.

Therefore, in summary, the design criteria for the surface water drainage system are as follows:

Return Period:	Minimum pipe full criteria	1 in 2 years
	Attenuation	1 in 30 years
	Flood Storage on site	1 in 100 years
	M5-60	17.700mm
	Ratio = M5-60/M5-2 day	0.270
	Limiting Discharge in storm event	5.4 l/s

Equivalent Impermeable Area Based on Modified Rational Method (Used in Microdrainage Software Design)			
Surface Type	Area (Ha)	Run-Off Coefficient (%)	Equiv. Imp. Area (Ha)
Roads	0.2834	100%	0.28
Footpath	0.1137	100%	0.11
On Curtilage Parking (Permeable Paving)	0.2707	100%	0.27
Roof Areas	0.6268	100%	0.63
Private Open space	0.422	0%	0.00
Public Open Space	0.4885	0%	0.00
Footpaths Draining onto Public Open Space	0.0769	0%	0.00
Total Site Area	2.282	Total Equivalent Impermeable Area (Ha)	1.29

Table 1 – Surface Types and Run-Off Coefficients based on the Modified Rational Method of Surface Water Design

The overall proposed surface water layout is shown on Drg. No. 21003-TJOC-ZZ-ZZ-DR-C-0064. The proposed MicroDrainage Surface Water Calculations for this application are provided in Appendix C. An allowance of 10% has been made in the calculations for an increase of rainfall due to climate change. This is consistent with Table 6.2 of Vol 2 of the GSDSDS.

The calculations indicates that no flooding of the proposed surface water network occurs during the critical 1 in 100 return period. The proposed Suds features, below ground drainage network and Stormtech attenuation system provides sufficient storage for the 1:30 year and the 1:100 critical storm events without flooding. A summary of the Surface water Storage and Attenuation Volumes generated by the proposed surface water network is shown in Table 2 below.

Revised Surface Water Storage/Attenuation Volumes		
Feature	Volume (M3)	Comments
Stormtech Attenuation MC3500	577	Located at east of site
Stormtech Attenuation SC740	136	Located at northwest of site
Piped Network and Manholes	96	
Tree Pits	18	Calculated based on 66.7m ³ of sub-surface storage medium with a minimum voids ratio of 30%. Volume excludes water that may infiltrate to ground if infiltration is available or water that is lost due to evapotranspiration
Filter Strips and Infiltration Trenches	62.7	Calculated based on 459 linear metres of Filter Strips and Infiltration Trenches. Volume excludes water that may infiltrate to ground if infiltration is available
Bioretention Area	11.8	Calculated based on 37m ² of Bioretention area
Subbase Storage in Permeable Paved Driveways and Car Park	211	Calculated based on 2707m ² of permeable paving across the site, average depth of water in subbase of 272mm and 30% Voids Ratio (2707m ² x 0.260m x 30% = 211 m ³)
Total Volume	1112.5	M3

Table 2 – Summary of Revised Surface Water Storage/Attenuation Volumes

The surface water discharge from the proposed development will be limited to 5.4l/s (equivalent to the Greenfield Run-off) by the use of Hydrobrake flow control device in MH S2 prior to discharging to the existing watercourse on the eastern boundary of the site.

It is proposed to install a Klargester petrol interceptor NSBE025 (by-pass separator) upstream of the final attenuation storage tank, to provide oil separation and storage for any possible risk of contamination and potential oil spills from the carpark/road area within the site.

It is proposed that the outfall pipe from the site will be set at a level of circa 250mm above the bed level of the adjacent watercourse. A non-return valve will be provided in an outfall/inspection chamber at MH S1 within the boundary of the site, refer to Drg. No. 21003-TJOC-ZZ-ZZ-DR-C-0073. The non-return valve in the outfall chamber will prevent backflow of water into the site surface water network during high water levels in the adjacent watercourse. The site surface water network and surface water storage measures within the site of the proposed development have been designed with freeboard and sufficient head within the system to allow the surface water to discharge from the site during high water levels in the adjacent watercourse, whilst complying with the requirement to retain surface water on site for critical storm events.

4. MAINTENANCE OF SUDS COMPONENTS

The inspection and maintenance of SUDS features and components will conform to the requirements of the SUDS Manual, CIRIA C753 (2015) and to the manufacturer's recommendations in respect of individual proprietary components. This information will be collated in the project Safety File and the operation and maintenance manual on completion of the project. A pack information will be provided to each homeowner to outline the SuDS features associated with their homes and the regular maintenance and monitoring that they will be required to carry out to ensure the systems operate as intended.

Maintenance operations will comprise the following categories:

- Establishment operations;
- Regular Maintenance;
- Occasional Maintenance; and
- Remedial Maintenance.

Establishment operations will include normal contractor responsibilities to ensure that planting becomes properly established. Also included in this category is the requirement to remove any construction stage silt from drainage pipework gullies etc and SuDS features on completion of a construction stage.

Regular maintenance activities will include:

- Inspection, including inspection of attenuation storage facilities which are proposed to occur monthly for first 3 months of operation and annually thereafter;
- Litter removal from the surface of the drainage catchment and all inlets to the drainage system (with potential to impact on performance) on a monthly basis;
- Grass cutting;
- Weed control;
- Shrub management;
- Vegetation management (in the context of the outfall area);

The Applicant commits to work alongside the Local Authority in developing long term maintenance strategies for any elements that will be taken in charge on completion of the works.

Occasional Maintenance will comprise silt removal, vegetation management and sweeping/brushing of permeable paved surfaces. While silt removal from gullies channels and catchpits will be undertaken on an as required basis (established on the basis of the routine inspections of infrastructure after initial construction), emptying of petrol interceptors will be in accordance with the manufacturer's

recommendations and will most be likely be at 12 monthly intervals. Sweeping/brushing of permeable paved surfaces will be undertaken on an annual basis typically.

Remedial maintenance activities would include any structural rehabilitation of drainage infrastructure and SUDS components and features. This could include raking out of joints in permeable paved surfaces and replacement of the joint material. It could also include the reconditioning of infiltration surfaces including the filter strips.


A detailed maintenance plan for the SuDS components will be developed as part of the detailed design of the works. However, Table 3 below sets out a typical Maintenance schedule for the SuDs Components proposed for this project.

Typical Maintenance Requirements for SuDS Components			
SuDS Component	Maintenance Type	Required Action	Typical Frequency
Permeable Paving	Regular	Brushing to remove leaves and debris that could fill/block joints in permeable paved surface	At least once yearly, but more occasionally as required
	Regular	Removal of weeds from joints in paving	At least once yearly, but more occasionally as required
	Regular	Remove Silt from Catchpits upstream of Permeable Paved areas.	Monitor for silt build-up and clean out catch pits as required
	Remedial Actions	Replace any broken paving slab and jointing material between slabs	As required
Infiltration Trenches	Regular	Remove Silt from Catchpits upstream of Infiltration Trenches.	Monitor for silt build-up and clean out catch pits as required
	Monitoring	Monitor for evidence of poor operation of SuDS component/slow release of surface water. Carry out routine maintenance to ensure silt build-up does not compromise the performance of the SuDS component	Every 3 months and before/after large storm events
Tree Pits	Regular	Remove Litter and Debris	Monthly or as required
	Regular	Inspect Inlets and Outlets for signs of silt build up/blockage	Inspect Monthly
	Occasional	Removal all silt build-up from inlets and surface and replace surface media as necessary	As required
	Monitoring	Record silt accumulation rates and establish appropriate frequency for removal of silt	Ongoing
Bioretention Area	Regular	Remove Litter and Debris. Replace any planting and maintain plant density within bioretention area	Monthly or as required
	Regular	Inspect Inlets and Outlets for signs of silt build up/blockage	Inspect Monthly
	Occasional	Removal all silt build-up from inlets and surface and replace surface media as necessary	As required
	Monitoring	Record extent of ponding within area and any silt accumulation. Establish appropriate frequency for removal of silt	Ongoing

Table 3 – Summary of Revised Surface Water Storage/Attenuation Volumes

APPENDIX A

Soakaway Test Results

 TJ O'Connor & Associates Corrig House Corrig Road Sandyford, Dublin 18	Project			Job Ref.	
	Gordon Park, Old Naas Road			21003	
	Soakaway Test Results			Sheet no./rev.	
	Test undertaken by	Date of Test	Weather at time of Test	Logged by	Calcs by
	Glide Construction and HSK	16/07/2021	Dry	Ciaran Sweeney	NS / TJOc

Soakaway Pit Dimensions	1.5	m long
	0.9	m wide
	1.3	m deep

S1_T3

Time [min]	Water Level [m]	% Full [%]	Water Depth [m]	Change [m]	Cumulative [m]
0	0	100%	1.3		0.00
10	0.1	80%	1.2	-0.1	0.10
20	0.2	73%	1.1	-0.1	0.20
30	0.26	69%	1.04	-0.06	0.26
40	0.34	64%	0.96	-0.08	0.34
50	0.42	59%	0.88	-0.08	0.42
60	0.49	54%	0.81	-0.07	0.49
70	0.54	51%	0.76	-0.05	0.54
80	0.61	46%	0.69	-0.07	0.61
90	0.67	42%	0.63	-0.06	0.67
100	0.71	39%	0.59	-0.04	0.71
110	0.75	37%	0.55	-0.04	0.75
120	0.8	33%	0.5	-0.05	0.80
130	0.84	31%	0.46	-0.04	0.84
140	0.88	28%	0.42	-0.04	0.88
150	0.93	25%	0.37	-0.05	0.93
160	0.95	23%	0.35	-0.02	0.95
170	1	20%	0.3	-0.05	1.00
180	1.04	17%	0.26	-0.04	1.04

Time [min]	Water Level [m]	% Full [%]	Water Depth [m]
0.00	0.00	100%	1.300
17.50	0.18	75%	1.125
71.43	0.55	50%	0.750
149.00	0.93	25%	0.375
	0.00	0%	1.300

Volume between 75% and 50%	$V_{p75-25} =$	1.02	m^3
Surface area at 50%	$a_{50} =$	4.95	m^2
Time between 75% and 25%	$t_{p75-25} =$	131.5	min
		2.19	hrs
Soil infiltration Rate	$f =$	2.6E-05	m/second
	$f =$	0.000026	m/second
		9.4E-02	m/hr
		0.094	m/hr

Notes:

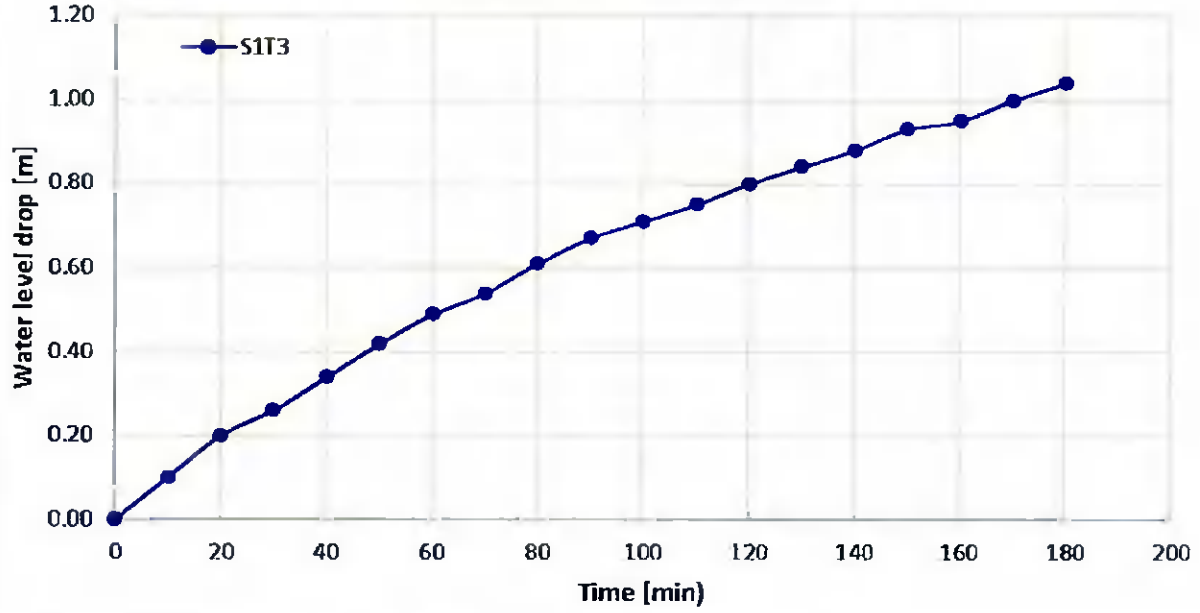
- Ground conditions – Brown Boulder Clay
- No Groundwater encountered
- Results based on 3rd test carried out at this pit.



TJ O'Connor & Associates
Corrig House
Corrig Road
Sandyford, Dublin 18

Project		Gordon Park, Old Naas Road		Job Ref.		21003			
Soakaway Test Results				Sheet no./rev.				2	
Test undertaken by		Date of Test		Weather at time of Test		Logged by		Calcs by	
Glide Construction and HSK		16/07/2021		Dry		Ciaran Sweeney		NS / TJOC	

Water Level Drop vs. Time





TJ O'Connor & Associates
 Corrig House
 Corrig Road
 Sandyford, Dublin 18


Project Gordon Park, Old Naas Road			Job Ref. 21003	
Soakaway Test Results			Sheet no./rev. 3	
Test undertaken by Glide Construction and HSK	Date of Test 16/07/2021	Weather at time of Test. Dry	Logged by Ciaran Sweeney	Calcs by NS / TJOC

Soakaway Pit Dimensions

S2_T1

1.5	m long
0.9	m wide
1.3	m deep

Time [min]	Water Level [m]	% Full [%]	Water Depth [m]	Change [m]	Cumulative [m]
0	0	100%	1.30		0.0
10	0.05	83%	1.25	-0.05	0.05
20	0.1	80%	1.20	-0.05	0.1
30	0.13	78%	1.17	-0.03	0.13
40	0.15	77%	1.15	-0.02	0.15
50	0.18	75%	1.12	-0.03	0.18
60	0.2	73%	1.10	-0.02	0.2
70	0.22	72%	1.08	-0.02	0.22
80	0.245	70%	1.06	-0.025	0.245
90	0.25	70%	1.05	-0.005	0.25
100	0.29	67%	1.01	-0.04	0.29
110	0.3	67%	1.00	-0.01	0.3
120	0.31	66%	0.99	-0.01	0.31
130	0.33	65%	0.97	-0.02	0.33
140	0.35	63%	0.95	-0.02	0.35
150	0.38	61%	0.92	-0.03	0.38
160	0.39	61%	0.91	-0.01	0.39
170	0.41	59%	0.89	-0.02	0.41
180	0.42	59%	0.88	-0.01	0.42
190	0.44	57%	0.86	-0.02	0.44
200	0.46	56%	0.84	-0.02	0.46
210	0.48	55%	0.82	-0.02	0.48
220	0.5	53%	0.80	-0.02	0.5
230	0.51	53%	0.79	-0.01	0.51
240	0.52	52%	0.78	-0.01	0.52
250	0.53	51%	0.77	-0.01	0.53
260	0.54	51%	0.76	-0.01	0.54
270	0.55	50%	0.75	-0.01	0.55
280	0.56	49%	0.74	-0.01	0.56
290	0.58	48%	0.72	-0.02	0.58
300	0.59	47%	0.71	-0.01	0.59
310	0.6	47%	0.70	-0.01	0.6
320	0.61	46%	0.69	-0.01	0.61
330	0.62	45%	0.68	-0.01	0.62
340	0.63	45%	0.67	-0.01	0.63
350	0.64	44%	0.66	-0.01	0.64
360	0.65	43%	0.65	-0.01	0.65
370	0.65	43%	0.65	0	0.65

 TJ O'Connor & Associates Corng House Corng Road Sandyford, Dublin 18	Project			Job Ref.	
	Gordon Park, Old Naas Road			21003	
	Soakaway Test Results			Sheet no./rev. 4	
Test undertaken by	Date of Test	Weather at time of Test		Logged by	Calcs by
Construction and HSK	16/07/2021	Dry		Ciaran Sweeney	NS / TJOC

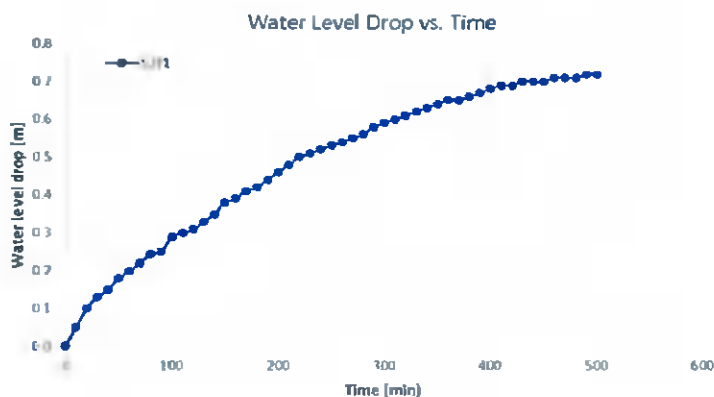
380	0.66	43%	0.64	-0.01	0.66
390	0.67	42%	0.63	-0.01	0.67
400	0.68	41%	0.62	-0.01	0.68
410	0.69	41%	0.61	-0.01	0.69
420	0.69	41%	0.61	0	0.69
430	0.7	40%	0.60	-0.01	0.7
440	0.7	40%	0.60	0	0.7
450	0.7	40%	0.60	0	0.7
460	0.71	39%	0.59	-0.01	0.71
470	0.71	39%	0.59	0	0.71
480	0.71	39%	0.59	0	0.71
490	0.72	39%	0.58	-0.01	0.72
500	0.72	39%	0.58	0	0.72

Time [min]	Water Level [m]	% Full [%]	Water Depth [m]	
0.00	0.00	100%	1.300	100.0
48.33	0.18	75%	1.125	75.0
270.00	0.55	50%	0.750	50.0
499.99	0.72	25%	0.581	38.7
	0.00	0%	1.300	0.0

Volume between 75% and 50%	$V_{p75-25} =$	0.74	m^3	
Surface area at 50%	$a_{s50} =$	4.95	m^2	
Time between 75 and 25%	$t_{p75-25} =$	451.7	min	7.53 hrs
Soil infiltration Rate	$f =$	5.5E-06	m/second	2.0E-02 m/hr
	$f =$	0.000006	m/second	0.020 m/hr

Notes:

- Ground conditions – Brown Boulder Clay
- No Groundwater encountered
- Poor infiltration recorded at this test pit.





TJ O'Connor & Associates
 Corrig House
 Corrig Road
 Sandyford, Dublin 18

Project Gordon Park, Old Naas Road			Job Ref. 21003	
Soakaway Test Results			Sheet no./rev. 5	
Test undertaken by Glide Construction and HSK	Date of Test. 16/07/2021	Weather at time of Test Dry	Logged by Ciaran Sweeney	Calcs by NS / TJOC

Soakaway Pit Dimensions

S3_T3

1.5	m long
0.9	m wide
1.3	m deep

Time [min]	Water Level [m]	% Full [%]	Water Depth [m]	Change [m]	Cumulative [m]
0	0	100%	1.30		0.0
10	0.22	72%	1.08	-0.22	0.22
20	0.3	67%	1.00	-0.08	0.3
30	0.38	61%	0.92	-0.08	0.38
40	0.45	57%	0.85	-0.07	0.45
50	0.53	51%	0.77	-0.08	0.53
60	0.59	47%	0.71	-0.06	0.59
70	0.64	44%	0.66	-0.05	0.64
80	0.69	41%	0.61	-0.05	0.69
90	0.72	39%	0.58	-0.03	0.72
100	0.75	37%	0.55	-0.03	0.75
110	0.79	34%	0.51	-0.04	0.79
120	0.82	32%	0.48	-0.03	0.82
130	0.86	29%	0.44	-0.04	0.86
140	0.89	27%	0.41	-0.03	0.89
150	0.92	25%	0.38	-0.03	0.92
160	0.94	24%	0.36	-0.02	0.94
170	0.97	22%	0.33	-0.03	0.97
180	1.01	19%	0.29	-0.04	1.01

Time [min]	Water Level [m]	% Full [%]	Water Depth [m]	Change [m]
0.00	0.00	100%	1.300	100
8.93	0.20	75%	1.104	75
53.33	0.55	50%	0.750	50
152.50	0.93	25%	0.375	25
	0.00	0%	1.300	0

Volume between 75% and 50%	$V_{p75-25} =$	0.99	m^3
Surface area at 50%	$a_{50} =$	4.95	m^2
Time between 75% and 25%	$t_{p75-25} =$	143.6	min
		2.39	hrs
Soil infiltration Rate	$f =$	2.3E-05	m/second
	$f =$	0.000023	m/second
		8.4E-02	m/hr
		0.084	m/hr

Notes:

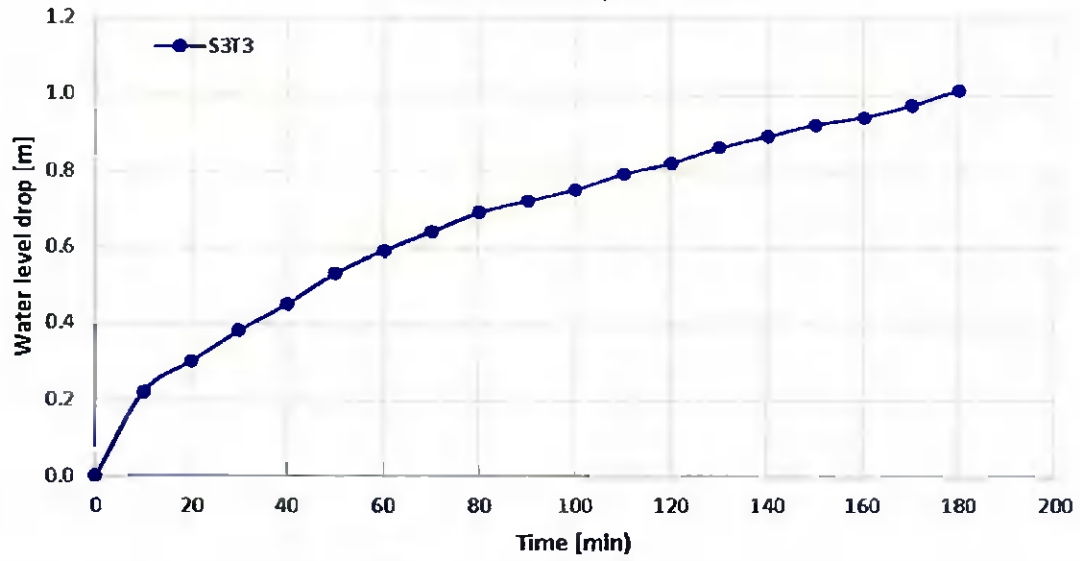
- Ground conditions – Brown Boulder Clay
- No Groundwater encountered
- Results based on 3rd test carried out at this pit.



TJ O'Connor & Associates
Corrig House
Corrig Road
Sandyford, Dublin 18

Project		Gordon Park, Old Naas Road		Job Ref.		21003			
Soakaway Test Results				Sheet no./rev.				6	
Test undertaken by		Date of Test		Weather at time of Test		Logged by		Calcs by	
Glide Construction and HSK		16/07/2021		Dry		Ciaran Sweeney		NS / TJOC	

Water Level Drop vs. Time



Project Gordon Park, Old Naas Road			Job Ref 21003	
Soakaway Test Results			Sheet no./rev. 7	
Test undertaken by Glide Construction and HSK	Date of Test 16/07/2021	Weather at time of Test Dry	Logged by Ciaran Sweeney	Calcs by NS / TJOC

Soakaway Pit Dimensions

S4_T1

1.5	m long
0.9	m wide
1.3	m deep

Time [min]	Water Level [m]	% Full [%]	Water Depth [m]	Change [m]	Cumulative [m]
0	0	100%	1.30		0.0
10	0.2	73%	1.10	-0.2	0.2
20	0.3	67%	1.00	-0.1	0.3
30	0.4	60%	0.90	-0.1	0.4
40	0.45	57%	0.85	-0.05	0.45
60	0.5	53%	0.80	-0.05	0.5
80	0.56	49%	0.74	-0.06	0.56
100	0.61	46%	0.69	-0.05	0.61
120	0.68	41%	0.62	-0.07	0.68
140	0.74	37%	0.56	-0.06	0.74
160	0.78	35%	0.52	-0.04	0.78
180	0.81	33%	0.49	-0.03	0.81
200	0.85	30%	0.45	-0.04	0.85
220	0.88	28%	0.42	-0.03	0.88
240	0.92	25%	0.38	-0.04	0.92
260	0.96	23%	0.34	-0.04	0.96

Time [min]	Water Level [m]	% Full [%]	Water Depth [m]	Change [m]
0.00	0.00	100%	1.300	100
9.38	0.19	75%	1.113	75
76.67	0.55	50%	0.750	50
242.50	0.93	25%	0.375	25
	0.00	0%	1.300	0

Volume between 75% and 50%	$V_{p75-25} =$	1.00	m^3
Surface area at 50%	$a_{s50} =$	4.95	m^2
Time between 75% and 25%	$t_{p75-25} =$	233.1	min 3.89 hrs
Soil infiltration Rate	$f =$	1.4E-05	m/second 5.2E-02 m/hr
	$f =$	0.000014	m/second 0.052 m/hr

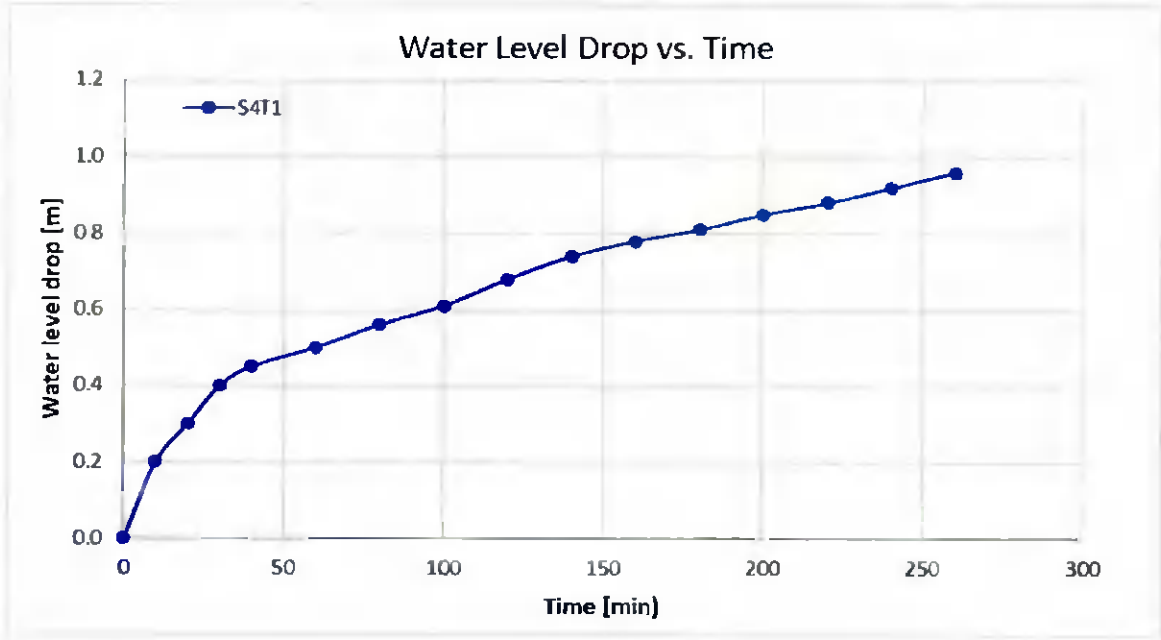
Notes:

- Ground conditions – Brown Boulder Clay
- No Groundwater encountered
- Poor infiltration recorded at this test pit.



TJ O'Connor & Associates
Corrig House
Corrig Road
Sandyford, Dublin 18

Project Gordon Park, Old Naas Road			Job Ref. 21003	
Soakaway Test Results			Sheet no./rev. 8	
Test undertaken by Glide Construction and HSK	Date of Test 16/07/2021	Weather at time of Test Dry	Logged by Ciaran Sweeney	Calcs by NS / TJOC





APPENDIX B

HR Wallingford Greenfield Runoff Report

Greenfield runoff rate estimation for sites

www.uksuds.com | Greenfield runoff tool

Calculated by:

Site name:

Site location:

Site Details

Latitude:

Longitude:

Reference:

Date:

This is an estimation of the greenfield runoff rates that are used to meet normal best practice criteria in line with Environment Agency guidance "Rainfall runoff management for developments", SC030219 (2013), the SuDS Manual C753 (Ciria, 2015) and the non-statutory standards for SuDS (Defra, 2015). This information on greenfield runoff rates may be the basis for setting consents for the drainage of surface water runoff from sites.

Runoff estimation approach

Site characteristics

Total site area (ha):

Methodology

Q_{BAR} estimation method:

SPR estimation method:

Soil characteristics

Default Edited

SOIL type:	<input type="text" value="2"/>	<input type="text" value="2"/>
HOST class:	<input type="text" value="N/A"/>	<input type="text" value="N/A"/>
SPR/SPRHOST:	<input type="text" value="0.3"/>	<input type="text" value="0.3"/>

Hydrological characteristics

Default Edited

SAAR (mm):	<input type="text" value="873"/>	<input type="text" value="873"/>
Hydrological region:	<input type="text" value="12"/>	<input type="text" value="12"/>
Growth curve factor 1 year:	<input type="text" value="0.85"/>	<input type="text" value="0.85"/>
Growth curve factor 30 years:	<input type="text" value="2.13"/>	<input type="text" value="2.13"/>
Growth curve factor 100 years:	<input type="text" value="2.61"/>	<input type="text" value="2.61"/>
Growth curve factor 200 years:	<input type="text" value="2.86"/>	<input type="text" value="2.86"/>

Notes

(1) Is Q_{BAR} < 2.0 l/s/ha?

When Q_{BAR} is < 2.0 l/s/ha then limiting discharge rates are set at 2.0 l/s/ha.

(2) Are flow rates < 5.0 l/s?

Where flow rates are less than 5.0 l/s consent for discharge is usually set at 5.0 l/s if blockage from vegetation and other materials is possible. Lower consent flow rates may be set where the blockage risk is addressed by using appropriate drainage elements.

(3) Is SPR/SPRHOST ≤ 0.3?

Where groundwater levels are low enough the use of soakaways to avoid discharge offsite would normally be preferred for disposal of surface water runoff.

Greenfield runoff rates

Default Edited

Q _{BAR} (l/s):	<input type="text" value="5.39"/>	<input type="text" value="5.39"/>
1 in 1 year (l/s):	<input type="text" value="4.58"/>	<input type="text" value="4.58"/>
1 in 30 years (l/s):	<input type="text" value="11.47"/>	<input type="text" value="11.47"/>
1 in 100 year (l/s):	<input type="text" value="14.06"/>	<input type="text" value="14.06"/>
1 in 200 years (l/s):	<input type="text" value="15.4"/>	<input type="text" value="15.4"/>

This report was produced using the greenfield runoff tool developed by HR Wallingford and available at www.uksuds.com. The use of this tool is subject to the UK SuDS terms and conditions and licence agreement, which can both be found at www.uksuds.com/terms-and-conditions.htm. The outputs from this tool are estimates of greenfield runoff rates. The use of these results is the responsibility of the users of this tool. No liability will be accepted by HR Wallingford, the Environment Agency, CEH, Hydrosolutions or any other organisation for the use of this data in the design or operational characteristics of any drainage scheme.

APPENDIX C

Proposed Surface Water Innovyze MicroDrainage Calculations

Corrig Hse Corrig Rd
 Sandyford Dublin 8
 Ireland
 Date 22/03/2022 14:19
 File Model 85.MDX
 CADS

Designed by Admin
 Checked by
 Network 2020.1.3



STORM SEWER DESIGN by the Modified Rational Method

Design Criteria for Storm

Pipe Sizes STANDARD Manhole Sizes STANDARD

FSR Rainfall Model - Scotland and Ireland

Return Period (years)	2	Maximum Rainfall (mm/hr)	50	Volumetric Runoff Coeff.	0.750	Minimum Backdrop Height (m)	0.200	Min Vel for Auto Design only (m/s)	1.00
M5-60 (mm)	17.700	Maximum Time of Concentration (mins)	30	PIMP (%)	100	Maximum Backdrop Height (m)	3.000	Min Slope for Optimisation (1:X)	500
Ratio R	0.270	Foul Sewage (l/s/ha)	0.000	Add Flow / Climate Change (%)	0	Min Design Depth for Optimisation (m)	1.200		

Designed with Level Soffits

Network Design Table for Storm

< - Indicates pipe capacity < flow

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	n	HYD SECT	DIA (mm)	Section Type	Auto Design
1.000	12.702	0.064	198.5	0.000	5.00	0.0	0.075	- _ -			Infiltration Trench	●
1.001	16.947	0.085	199.4	0.000	0.00	0.0	0.600		o	150	Pipe/Conduit	●
1.002	14.385	0.240	59.9	0.000	0.00	0.0	0.600		o	150	Pipe/Conduit	●
2.000	4.975	0.025	199.0	0.000	5.00	0.0	0.075	- _ -			Porous Car Park	●
2.001	6.420	0.107	60.0	0.000	0.00	0.0	0.600		o	150	Pipe/Conduit	●
3.000	4.756	0.024	198.2	0.000	5.00	0.0	0.075	- _ -			Porous Car Park	●
3.001	10.755	0.179	60.1	0.000	0.00	0.0	0.600		o	150	Pipe/Conduit	●
1.003	19.978	0.100	199.8	0.021	0.00	0.0	0.600		o	225	Pipe/Conduit	●
4.000	21.334	0.107	199.4	0.023	5.00	0.0	0.600		o	225	Pipe/Conduit	●
4.001	7.901	0.040	197.5	0.000	0.00	0.0	0.600		o	225	Pipe/Conduit	●
5.000	6.877	0.034	202.3	0.000	5.00	0.0	0.075	- _ -			Porous Car Park	●
5.001	10.286	0.171	60.2	0.000	0.00	0.0	0.075		o	150	Pipe/Conduit	●
6.000	2.500	0.013	192.3	0.000	5.00	0.0	0.075	- _ -			Porous Car Park	●

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	E I.Area (ha)	E Base Flow (l/s)	Foul Flow (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
1.000	46.85	6.77	94.000	0.000	0.0	0.0	0.0	0.12	12.9	0.0
1.001	45.74	7.17	93.886	0.000	0.0	0.0	0.0	0.71	12.5	0.0
1.002	45.25	7.35	93.801	0.000	0.0	0.0	0.0	1.30	23.0	0.0
2.000	50.00	5.70	94.190	0.000	0.0	0.0	0.0	0.12	205.6	0.0
2.001	49.90	5.78	94.165	0.000	0.0	0.0	0.0	1.30	23.0	0.0
3.000	50.00	5.60	94.190	0.000	0.0	0.0	0.0	0.13	288.7	0.0
3.001	50.00	5.74	94.166	0.000	0.0	0.0	0.0	1.30	23.0	0.0
1.003	44.33	7.71	93.486	0.021	0.0	0.0	0.0	0.92	36.6	2.5
4.000	50.00	5.39	92.306	0.023	0.0	0.0	0.0	0.92	36.7	3.1
4.001	50.00	5.53	92.199	0.023	0.0	0.0	0.0	0.93	36.8	3.1
5.000	49.24	5.98	94.640	0.000	0.0	0.0	0.0	0.12	137.7	0.0
5.001	46.55	6.87	94.606	0.000	0.0	0.0	0.0	0.19	3.4	0.0
6.000	50.00	5.35	94.665	0.000	0.0	0.0	0.0	0.12	142.4	0.0

Corrig Hse Corrig Rd
 Sandyford Dublin 8
 Ireland
 Date 22/03/2022 14:19
 File Model 65.MDX
 CADS

Designed by Admin
 Checked by
 Network 2020.1.3



Network Design Table for Storm

EN	Length (m)	Fall (m)	Slope (1:K)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	n	HYD SECT	DIA (mm)	Section Type	Auto Design
6.001	7.206	0.127	56.7	0.000	0.00	0.0		0.075	o	150	Pipe/Conduit	●
1.004	22.873	0.114	200.6	0.028	0.00	0.0	0.600		o	225	Pipe/Conduit	●
7.000	5.100	0.026	196.2	0.000	5.00	0.0		0.075	- -		Porous Car Park	●
7.001	5.293	0.088	60.1	0.000	0.00	0.0		0.075	o	150	Pipe/Conduit	●
8.000	22.568	0.113	199.7	0.000	5.00	0.0		0.075	- -		Infiltration Trench	●
8.001	26.567	0.133	199.8	0.000	0.00	0.0		0.075	o	150	Pipe/Conduit	●
1.005	9.608	0.048	200.2	0.000	0.00	0.0		0.075	o	225	Pipe/Conduit	●
1.006	15.627	0.078	200.3	0.030	0.00	0.0		0.075	o	225	Pipe/Conduit	●
9.000	5.200	0.026	200.0	0.000	5.00	0.0		0.075	- -		Porous Car Park	●
9.001	5.800	0.097	59.8	0.000	0.00	0.0		0.075	o	150	Pipe/Conduit	●
10.000	22.836	0.114	200.3	0.000	5.00	0.0		0.075	- -		Infiltration Trench	●
10.001	27.438	0.137	200.3	0.000	0.00	0.0		0.075	o	225	Pipe/Conduit	●
1.007	10.126	0.051	198.5	0.000	0.00	0.0		0.075	o	225	Pipe/Conduit	●
11.000	5.200	0.026	200.0	0.000	5.00	0.0		0.075	- -		Porous Car Park	●
11.001	6.000	0.100	60.0	0.000	0.00	0.0		0.075	o	150	Pipe/Conduit	●
1.008	9.068	0.045	201.5	0.000	0.00	0.0	0.600		o	225	Pipe/Conduit	●
12.000	25.661	0.128	200.5	0.000	5.00	0.0		0.075	- -		Infiltration Trench	●
12.001	22.481	0.112	200.7	0.000	0.00	0.0	0.600		o	150	Pipe/Conduit	●

Network Results Table

EN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	E I.Area (ha)	E Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
6.001	49.33	5.95	94.653	0.000	0.0	0.0	0.0	0.20	3.5	0.0
1.004	43.33	8.13	92.160	0.072	0.0	0.0	0.0	0.92	36.6	8.4
7.000	50.00	5.71	94.440	0.000	0.0	0.0	0.0	0.12	201.6	0.0
7.001	48.64	6.17	94.414	0.000	0.0	0.0	0.0	0.19	3.4	0.0
8.000	44.25	7.74	94.273	0.000	0.0	0.0	0.0	0.14	19.3	0.0
8.001	36.21	11.93	94.160	0.000	0.0	0.0	0.0	0.11	1.9	0.0
1.005	34.58	13.09	92.046	0.072	0.0	0.0	0.0	0.14	5.5x	8.4
1.006	32.28	14.97	91.998	0.102	0.0	0.0	0.0	0.14	5.5x	8.9
9.000	50.00	5.73	94.190	0.000	0.0	0.0	0.0	0.12	253.4	0.0
9.001	48.44	6.23	94.164	0.000	0.0	0.0	0.0	0.19	3.4	0.0
10.000	43.91	7.88	94.000	0.000	0.0	0.0	0.0	0.13	16.6	0.0
10.001	37.37	11.19	93.886	0.000	0.0	0.0	0.0	0.14	5.5	0.0
1.007	31.00	16.19	91.920	0.102	0.0	0.0	0.0	0.14	5.5x	8.9
11.000	50.00	5.73	94.040	0.000	0.0	0.0	0.0	0.12	288.8	0.0
11.001	48.39	6.25	94.014	0.000	0.0	0.0	0.0	0.19	3.4	0.0
1.008	30.84	16.35	91.869	0.102	0.0	0.0	0.0	0.92	36.5	8.9
12.000	43.06	8.24	93.850	0.000	0.0	0.0	0.0	0.13	16.6	0.0
12.001	41.88	8.77	93.722	0.000	0.0	0.0	0.0	0.71	12.5	0.0

Corrig Hse Corrig Rd
 Sandyford Dublin 8
 Ireland
 Date 22/03/2022 14:19
 File Model 85.MDX
 CADS

Designed by Admin
 Checked by
 Network 2020.1.3



Network Design Table for Storm

PN	Length (m)	Fall (1:X)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	n	HYD SECT	DIA (mm)	Section Type	Auto Design
12.002	18.920	0.095	199.2	0.000	0.00	0.0	0.600		o	150	Pipe/Conduit	●
1.009	6.046	0.030	201.5	0.000	0.00	0.0	0.600		o	225	Pipe/Conduit	●
13.000	6.000	0.030	200.0	0.000	5.00	0.0		0.075	- 1 -		Porous Car Park	●
13.001	16.200	0.270	60.0	0.000	0.00	0.0	0.600		o	150	Pipe/Conduit	●
1.010	22.964	0.115	199.7	0.030	0.00	0.0	0.600		o	225	Pipe/Conduit	●
14.000	11.533	0.058	198.8	0.000	5.00	0.0		0.075	- 1 -		Infiltration Trench	●
14.001	16.960	0.085	199.5	0.000	0.00	0.0	0.600		o	150	Pipe/Conduit	●
14.002	13.994	0.070	199.9	0.000	0.00	0.0	0.600		o	150	Pipe/Conduit	●
1.011	7.237	0.036	201.0	0.000	0.00	0.0	0.600		o	225	Pipe/Conduit	●
1.012	7.200	0.036	200.0	0.000	0.00	0.0	0.600		o	225	Pipe/Conduit	●
15.000	5.232	0.026	201.2	0.000	5.00	0.0		0.075	- 1 -		Porous Car Park	●
15.001	5.400	0.090	60.0	0.000	0.00	0.0	0.600		o	150	Pipe/Conduit	●
16.000	6.221	0.031	200.7	0.000	5.00	0.0		0.075	- 1 -		Porous Car Park	●
16.001	9.359	0.156	60.0	0.000	0.00	0.0	0.600		o	150	Pipe/Conduit	●
15.002	25.983	0.130	199.9	0.030	0.00	0.0	0.600		o	225	Pipe/Conduit	●
17.000	6.952	0.035	198.6	0.000	5.00	0.0		0.075	- 1 -		Porous Car Park	●
17.001	8.258	0.138	59.8	0.000	0.00	0.0	0.600		o	150	Pipe/Conduit	●
18.000	20.311	0.102	199.1	0.000	5.00	0.0		0.075	- 1 -		Infiltration Trench	●

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	E I.Area (ha)	E Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
12.002	40.95	9.22	93.610	0.000	0.0	0.0	0.0	0.71	12.5	0.0
1.009	30.74	16.46	91.824	0.102	0.0	0.0	0.0	0.92	36.5	8.9
13.000	49.67	5.85	93.890	0.000	0.0	0.0	0.0	0.12	128.9	0.0
13.001	48.99	6.06	93.860	0.000	0.0	0.0	0.0	1.30	23.0	0.0
1.010	30.34	16.88	91.794	0.132	0.0	0.0	0.0	0.92	36.6	10.6
14.000	47.31	6.61	93.550	0.000	0.0	0.0	0.0	0.12	12.9	0.0
14.001	46.18	7.01	93.492	0.000	0.0	0.0	0.0	0.71	12.5	0.0
14.002	45.29	7.34	93.407	0.000	0.0	0.0	0.0	0.71	12.5	0.0
1.011	30.22	17.01	91.679	0.132	0.0	0.0	0.0	0.92	36.5	10.6
1.012	30.10	17.14	91.643	0.132	0.0	0.0	0.0	0.92	36.6	10.6
15.000	50.00	5.75	93.965	0.000	0.0	0.0	0.0	0.12	124.2	0.0
15.001	49.79	5.81	93.939	0.000	0.0	0.0	0.0	1.30	23.0	0.0
16.000	49.57	5.88	93.965	0.000	0.0	0.0	0.0	0.12	201.5	0.0
16.001	49.18	6.00	93.934	0.000	0.0	0.0	0.0	1.30	23.0	0.0
15.002	47.72	6.47	92.663	0.030	0.0	0.0	0.0	0.92	36.6	3.9
17.000	49.23	5.98	93.740	0.000	0.0	0.0	0.0	0.12	130.4	0.0
17.001	48.90	6.09	93.555	0.000	0.0	0.0	0.0	1.30	23.0	0.0
18.000	44.52	7.63	93.702	0.000	0.0	0.0	0.0	0.13	14.8	0.0

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Network Design Table for Storm

FN	Length (m)	Fall (m)	Slope (1:K)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	n	HYD SECT	DIA (mm)	Section Type	Auto Design
18.001	12.993	0.065	199.9	0.000	0.00	0.0		0.075	- _ -		Infiltration Trench	●
18.002	16.730	0.084	199.2	0.000	0.00	0.0	0.600		o	150	Pipe/Conduit	●
18.003	15.344	0.077	199.3	0.000	0.00	0.0	0.600		o	150	Pipe/Conduit	●
15.003	20.391	0.102	199.9	0.033	0.00	0.0	0.600		o	225	Pipe/Conduit	●
19.000	17.678	0.227	77.9	0.000	5.00	0.0		0.075	- _ -		Infiltration Trench	●
19.001	12.792	0.128	99.9	0.000	0.00	0.0		0.075	- _ -		Infiltration Trench	●
19.002	17.417	0.290	60.1	0.000	0.00	0.0		0.075	o	150	Pipe/Conduit	●
19.003	6.458	0.065	99.4	0.000	0.00	0.0		0.075	o	150	Pipe/Conduit	●
19.004	13.276	0.027	491.7	0.000	0.00	0.0		0.035	-\ /-		Infiltration Basin	●
19.005	5.446	0.027	201.7	0.000	0.00	0.0		0.075	o	225	Pipe/Conduit	●
19.006	5.379	0.027	199.2	0.000	0.00	0.0		0.075	o	225	Pipe/Conduit	●
20.000	6.570	0.033	199.1	0.000	5.00	0.0		0.075	- _ -		Porous Car Park	●
20.001	12.739	0.212	60.1	0.000	0.00	0.0	0.600		o	150	Pipe/Conduit	●
15.004	51.961	0.260	199.9	0.022	0.00	0.0	0.600		o	225	Pipe/Conduit	●
15.005	12.244	0.061	200.7	0.000	0.00	0.0	0.600		o	225	Pipe/Conduit	●
15.006	9.948	0.050	199.0	0.022	0.00	0.0	0.600		o	225	Pipe/Conduit	●
21.000	16.200	0.081	200.0	0.000	5.00	0.0		0.075	- _ -		Infiltration Trench	●
21.001	8.203	0.041	200.1	0.000	0.00	0.0		0.075	- _ -		Infiltration Trench	●
21.002	15.243	0.076	200.6	0.000	0.00	0.0		0.075	- _ -		Infiltration Trench	●
21.003	7.611	0.038	200.3	0.000	0.00	0.0	0.600		o	150	Pipe/Conduit	●
15.007	4.097	0.020	204.9	0.015	0.00	0.0	0.600		o	225	Pipe/Conduit	●
15.008	5.743	0.029	198.0	0.000	0.00	0.0	0.600		o	225	Pipe/Conduit	●

Network Results Table

FN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	I.Area (ha)	E Base Flow (l/s)	Foul Flow (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
18.001	40.83	9.27	93.550	0.000	0.0	0.0	0.0	0.13	16.7	0.0
18.002	40.05	9.67	93.435	0.000	0.0	0.0	0.0	0.71	12.5	0.0
18.003	39.37	10.03	93.351	0.000	0.0	0.0	0.0	0.71	12.5	0.0
15.003	38.71	10.40	92.533	0.063	0.0	0.0	0.0	0.92	36.6	6.6
19.000	47.96	6.39	94.000	0.000	0.0	0.0	0.0	0.21	26.7	0.0
19.001	44.99	7.45	93.619	0.000	0.0	0.0	0.0	0.20	31.9	0.0
19.002	41.48	8.96	93.267	0.000	0.0	0.0	0.0	0.19	3.4	0.0
19.003	40.03	9.68	92.977	0.000	0.0	0.0	0.0	0.15	2.6	0.0
19.004	39.25	10.09	92.912	0.000	0.0	0.0	0.0	0.53	403.4	0.0
19.005	38.09	10.75	92.485	0.000	0.0	0.0	0.0	0.14	5.5	0.0
19.006	37.03	11.40	92.458	0.000	0.0	0.0	0.0	0.14	5.5	0.0
20.000	49.43	5.92	93.485	0.000	0.0	0.0	0.0	0.12	227.1	0.0
20.001	48.90	6.09	93.452	0.000	0.0	0.0	0.0	1.30	23.0	0.0
15.004	35.61	12.34	92.431	0.085	0.0	0.0	0.0	0.92	36.6	8.2
15.005	35.30	12.56	92.171	0.085	0.0	0.0	0.0	0.92	36.5	8.2
15.006	35.05	12.74	92.110	0.107	0.0	0.0	0.0	0.92	36.7	10.2
21.000	46.08	7.04	93.295	0.000	0.0	0.0	0.0	0.13	16.7	0.0
21.001	43.59	8.01	93.164	0.000	0.0	0.0	0.0	0.14	21.6	0.0
21.002	39.84	9.77	93.073	0.000	0.0	0.0	0.0	0.14	24.2	0.0
21.003	39.51	9.95	92.562	0.000	0.0	0.0	0.0	0.71	12.5	0.0
15.007	34.95	12.82	92.060	0.122	0.0	0.0	0.0	0.91	36.2	11.5
15.008	34.81	12.92	92.040	0.122	0.0	0.0	0.0	0.93	36.8	11.5

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Network Design Table for Storm

FN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	n	HYD SECT	DIA (mm)	Section Type	Auto Design
22.000	17.000	0.034	500.0	0.000	5.00	0.0	0.075	- 1			Porous Car Park	●
22.001	6.700	0.034	197.1	0.000	0.00	0.0	0.600		o	225	Pipe/Conduit	●
22.002	10.592	0.053	199.8	0.000	0.00	0.0	0.600		o	225	Pipe/Conduit	●
23.000	5.400	0.027	200.0	0.000	5.00	0.0	0.075	- 1			Porous Car Park	●
23.001	9.512	0.159	59.8	0.000	0.00	0.0	0.600		o	150	Pipe/Conduit	●
23.002	9.647	0.048	201.0	0.000	0.00	0.0	0.600		o	225	Pipe/Conduit	●
24.000	6.750	0.034	198.5	0.000	5.00	0.0	0.075	- 1			Porous Car Park	●
24.001	8.660	0.144	60.1	0.000	0.00	0.0	0.600		o	150	Pipe/Conduit	●
24.002	8.459	0.042	201.4	0.000	0.00	0.0	0.600		o	225	Pipe/Conduit	●
25.000	12.500	0.025	500.0	0.000	5.00	0.0	0.600		o	100	Pipe/Conduit	●
23.003	18.547	0.093	199.4	0.000	0.00	0.0	0.600		o	225	Pipe/Conduit	●
26.000	22.748	0.114	199.5	0.000	5.00	0.0	0.600		o	225	Pipe/Conduit	●
26.001	22.445	0.112	200.4	0.000	0.00	0.0	0.600		o	225	Pipe/Conduit	●
26.002	15.387	0.077	199.8	0.000	0.00	0.0	0.600		o	225	Pipe/Conduit	●
27.000	18.014	0.090	200.2	0.000	5.00	0.0	0.600		o	225	Pipe/Conduit	●
27.001	14.719	0.213	69.1	0.000	0.00	0.0	0.600		o	225	Pipe/Conduit	●
26.003	12.980	0.065	199.7	0.000	0.00	0.0	0.600		o	225	Pipe/Conduit	●
22.003	3.500	0.018	194.4	0.000	0.00	0.0	0.600		o	225	Pipe/Conduit	●
22.004	28.142	0.048	586.3	0.045	0.00	0.0	0.050	- 1			Cellular Storage	●

Network Results Table

FN	Rain (mm/hr)	T.C. (mins)	US/XL (m)	E I.Area (ha)	E Base Flow (l/s)	Foul Flow (l/s)	Add Flow (l/s)	Flow Vel (m/s)	Cap (l/s)	Flow (l/s)
22.000	41.55	8.93	93.340	0.000	0.0	0.0	0.0	0.07	148.8	0.0
22.001	41.29	9.05	93.346	0.000	0.0	0.0	0.0	0.93	36.9	0.0
22.002	40.90	9.24	92.148	0.000	0.0	0.0	0.0	0.92	36.6	0.0
23.000	49.97	5.76	93.740	0.000	0.0	0.0	0.0	0.12	255.5	0.0
23.001	49.57	5.88	93.713	0.000	0.0	0.0	0.0	1.30	23.0	0.0
23.002	49.00	6.06	92.236	0.000	0.0	0.0	0.0	0.92	36.5	0.0
24.000	49.35	5.95	93.500	0.000	0.0	0.0	0.0	0.12	201.5	0.0
24.001	48.99	6.06	93.466	0.000	0.0	0.0	0.0	1.30	23.0	0.0
24.002	48.50	6.21	92.230	0.000	0.0	0.0	0.0	0.92	36.5	0.0
25.000	50.00	5.62	92.700	0.000	0.0	0.0	0.0	0.34	2.7	0.0
23.003	47.48	6.55	92.188	0.000	0.0	0.0	0.0	0.92	36.7	0.0
26.000	50.00	5.41	93.340	0.000	0.0	0.0	0.0	0.92	36.7	0.0
26.001	49.78	5.82	93.226	0.000	0.0	0.0	0.0	0.92	36.6	0.0
26.002	48.87	6.10	93.114	0.000	0.0	0.0	0.0	0.92	36.6	0.0
27.000	50.00	5.33	93.340	0.000	0.0	0.0	0.0	0.92	36.6	0.0
27.001	50.00	5.48	93.250	0.000	0.0	0.0	0.0	1.58	62.6	0.0
26.003	48.14	6.33	93.037	0.000	0.0	0.0	0.0	0.92	36.6	0.0
22.003	40.77	9.30	92.095	0.000	0.0	0.0	0.0	0.93	37.1	0.0
22.004	38.75	10.37	92.078	0.045	0.0	0.0	0.0	0.44	2110.4	4.7

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Network Design Table for Storm

FN	Length (m)	Fall (m)	Slope (1:K)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	n	HYD SECT	DIA (mm)	Section Type	Auto Design
22.005	3.605	0.018	200.3	0.000	0.00	0.0	0.050	o	225		Pipe/Conduit	●
15.009	9.910	0.050	198.2	0.000	0.00	0.0	0.600	o	225		Pipe/Conduit	●
15.010	4.285	0.021	204.0	0.000	0.00	0.0	0.600	o	225		Pipe/Conduit	●
15.011	12.967	0.065	199.5	0.030	0.00	0.0	0.600	o	225		Pipe/Conduit	●
15.012	6.043	0.030	201.4	0.000	0.00	0.0	0.600	o	225		Pipe/Conduit	●
28.000	22.392	0.112	199.9	0.000	5.00	0.0	0.075	- _ -			Infiltration Trench	●
28.001	13.233	0.066	200.5	0.000	0.00	0.0	0.075	- _ -			Infiltration Trench	●
28.002	12.869	0.064	201.1	0.000	0.00	0.0	0.075	o	150		Pipe/Conduit	●
15.013	6.306	0.032	197.1	0.000	0.00	0.0	0.600	o	225		Pipe/Conduit	●
29.000	5.000	0.025	200.0	0.000	5.00	0.0	0.075	- _ -			Porous Car Park	●
29.001	11.100	0.185	60.0	0.000	0.00	0.0	0.600	o	150		Pipe/Conduit	●
30.000	17.857	0.089	200.6	0.000	5.00	0.0	0.075	- _ -			Infiltration Trench	●
30.001	17.475	0.087	200.9	0.000	0.00	0.0	0.075	- _ -			Infiltration Trench	●
30.002	13.076	0.065	201.2	0.000	0.00	0.0	0.075	o	150		Pipe/Conduit	●
31.000	20.489	0.102	200.9	0.000	5.00	0.0	0.075	- _ -			Infiltration Trench	●
32.000	15.560	0.078	199.5	0.000	5.00	0.0	0.075	- _ -			Infiltration Trench	●
31.001	16.943	0.085	199.3	0.000	0.00	0.0	0.075	o	150		Pipe/Conduit	●
31.002	13.606	0.068	200.1	0.000	0.00	0.0	0.075	o	150		Pipe/Conduit	●
15.014	21.973	0.110	199.8	0.028	0.00	0.0	0.600	o	225		Pipe/Conduit	●

Network Results Table

FN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
22.005	38.24	10.66	92.030	0.045	0.0	0.0	0.0	0.21	8.2	4.7
15.009	34.57	13.10	92.011	0.167	0.0	0.0	0.0	0.93	36.8	15.6
15.010	34.46	13.18	91.961	0.167	0.0	0.0	0.0	0.91	36.2	15.6
15.011	34.16	13.41	91.940	0.197	0.0	0.0	0.0	0.92	36.7	18.2
15.012	34.02	13.52	91.875	0.197	0.0	0.0	0.0	0.92	36.5	18.2
28.000	44.06	7.82	93.550	0.000	0.0	0.0	0.0	0.13	16.7	0.0
28.001	40.47	9.45	93.310	0.000	0.0	0.0	0.0	0.14	18.5	0.0
28.002	36.89	11.49	92.472	0.000	0.0	0.0	0.0	0.11	1.9	0.0
15.013	33.87	13.63	91.845	0.197	0.0	0.0	0.0	0.93	36.9	18.2
29.000	50.00	5.71	93.590	0.000	0.0	0.0	0.0	0.12	158.9	0.0
29.001	49.67	5.85	93.565	0.000	0.0	0.0	0.0	1.30	23.0	0.0
30.000	45.50	7.26	93.735	0.000	0.0	0.0	0.0	0.13	16.6	0.0
30.001	40.63	9.37	93.285	0.000	0.0	0.0	0.0	0.14	19.8	0.0
30.002	36.96	11.44	92.517	0.000	0.0	0.0	0.0	0.11	1.9	0.0
31.000	44.73	7.55	93.424	0.000	0.0	0.0	0.0	0.13	17.6	0.0
32.000	46.31	6.96	93.400	0.000	0.0	0.0	0.0	0.13	16.7	0.0
31.001	39.02	10.22	93.272	0.000	0.0	0.0	0.0	0.11	1.9	0.0
31.002	35.57	12.37	93.187	0.000	0.0	0.0	0.0	0.11	1.9	0.0
15.014	33.38	14.03	91.813	0.225	0.0	0.0	0.0	0.92	36.6	20.3

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Network Design Table for Storm

FN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	n	BYD SECT	DIA (mm)	Section Type	Auto Design
33.000	5.600	0.028	200.0	0.000	5.00	0.0	0.075	-[1]-			Porous Car Park	●
33.001	10.500	0.175	60.0	0.000	0.00	0.0	0.600	o	150		Pipe/Conduit	●
15.015	6.467	0.032	202.1	0.000	0.00	0.0	0.600	o	225		Pipe/Conduit	●
34.000	5.200	0.026	200.0	0.000	5.00	0.0	0.075	-[1]-			Porous Car Park	●
34.001	5.900	0.098	60.2	0.000	0.00	0.0	0.600	o	150		Pipe/Conduit	●
34.002	44.743	0.224	199.7	0.026	0.00	0.0	0.600	o	225		Pipe/Conduit	●
35.000	5.200	0.025	208.0	0.000	5.00	0.0	0.075	-[1]-			Porous Car Park	●
35.001	6.500	0.108	60.2	0.000	0.00	0.0	0.075	o	150		Pipe/Conduit	●
36.000	18.820	0.094	200.2	0.000	5.00	0.0	0.075	-[1]-			Infiltration Trench	●
36.001	17.126	0.086	199.1	0.000	0.00	0.0	0.075	o	150		Pipe/Conduit	●
36.002	13.516	0.068	198.8	0.000	0.00	0.0	0.075	o	150		Pipe/Conduit	●
15.016	6.287	0.031	202.8	0.015	0.00	0.0	0.600	o	225		Pipe/Conduit	●
37.000	5.740	0.029	197.9	0.000	5.00	0.0	0.075	-[1]-			Porous Car Park	●
37.001	11.632	0.058	200.6	0.000	0.00	0.0	0.600	o	150		Pipe/Conduit	●
15.017	7.021	0.035	200.6	0.000	0.00	0.0	0.600	o	225		Pipe/Conduit	●
1.013	3.017	0.006	502.8	0.000	0.00	0.0	0.600	o	225		Pipe/Conduit	●
1.014	24.822	0.050	496.4	0.000	0.00	0.0	0.050	-[1]			Cellular Storage	●
1.015	4.410	0.022	200.5	0.000	0.00	0.0	0.600	o	225		Pipe/Conduit	●

Network Results Table

FN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	E I.Area (ha)	E Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Val (m/s)	Cap (l/s)	Flow (l/s)
33.000	49.87	5.79	93.640	0.000	0.0	0.0	0.0	0.12	208.3	0.0
33.001	49.43	5.92	93.612	0.000	0.0	0.0	0.0	1.30	23.0	0.0
15.015	33.24	14.15	91.703	0.225	0.0	0.0	0.0	0.92	36.4	20.3
34.000	50.00	5.73	93.925	0.000	0.0	0.0	0.0	0.12	198.6	0.0
34.001	49.81	5.81	93.899	0.000	0.0	0.0	0.0	1.30	22.9	0.0
34.002	47.28	6.62	93.135	0.026	0.0	0.0	0.0	0.92	36.6	3.3
35.000	50.00	5.75	93.475	0.000	0.0	0.0	0.0	0.12	271.6	0.0
35.001	48.21	6.31	93.200	0.000	0.0	0.0	0.0	0.19	3.4	0.0
36.000	45.19	7.37	93.550	0.000	0.0	0.0	0.0	0.13	16.6	0.0
36.001	39.29	10.07	93.406	0.000	0.0	0.0	0.0	0.11	1.9	0.0
36.002	35.82	12.20	93.320	0.000	0.0	0.0	0.0	0.11	1.9	0.0
15.016	33.10	14.26	91.671	0.266	0.0	0.0	0.0	0.91	36.4	23.8
37.000	49.82	5.81	93.740	0.000	0.0	0.0	0.0	0.12	193.2	0.0
37.001	48.92	6.08	93.711	0.000	0.0	0.0	0.0	0.71	12.5	0.0
15.017	32.95	14.39	91.640	0.266	0.0	0.0	0.0	0.92	36.6	23.8
1.013	30.02	17.23	91.607	0.398	0.0	0.0	0.0	0.58	22.9*	32.4
1.014	29.42	17.91	91.601	0.398	0.0	0.0	0.0	0.61	13968.8	32.4
1.015	29.35	17.99	91.447	0.398	0.0	0.0	0.0	0.92	36.6	32.4

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PIPELINE SCHEDULES for Storm

Upstream Manhole

PN	Byd Sect	Diam (mm)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MB DIAM., L*W (mm)
1.000	- _1-		CP15	94.600	94.000	0.000	Open Manhole	600
1.001	o 150		CP14	94.600	93.886	0.564	Open Manhole	600
1.002	o 150		AJ13	94.400	93.801	0.449	Open Manhole	600
2.000	- _1-		Dr23	94.600	94.190	0.110	Open Manhole	600
2.001	o 150		Dr24	94.600	94.165	0.285	Open Manhole	600
3.000	- _1-		Dr25	94.600	94.190	0.110	Open Manhole	600
3.001	o 150		Dr26	94.600	94.166	0.284	Open Manhole	600
1.003	o 225		S11	94.558	93.486	0.847	Open Manhole	1200
4.000	o 225		S17	95.501	92.306	2.970	Open Manhole	1200
4.001	o 225		S16	94.954	92.199	2.530	Open Manhole	1200
5.000	- _1-		Dr27	95.050	94.640	0.110	Open Manhole	600
5.001	o 150		Dr28	95.050	94.606	0.294	Open Manhole	600
6.000	- _1-		Dr29	95.075	94.665	0.110	Open Manhole	600
6.001	o 150		Dr30	95.075	94.653	0.272	Open Manhole	600
1.004	o 225		S10	94.783	92.160	2.398	Open Manhole	1200
7.000	- _1-		Dr31	94.850	94.440	0.110	Open Manhole	600
7.001	o 150		Dr32	94.850	94.414	0.286	Open Manhole	600
8.000	- _1-		CP20	95.075	94.273	0.132	Open Manhole	600

Downstream Manhole

PN	Length (m)	Slope (1:X)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MB DIAM., L*W (mm)
1.000	12.702	198.5	CP14	94.600	93.936	0.064	Open Manhole	600
1.001	16.947	199.4	AJ13	94.400	93.801	0.449	Open Manhole	600
1.002	14.385	59.9	S11	94.558	93.561	0.847	Open Manhole	1200
2.000	4.975	199.0	Dr24	94.600	94.165	0.135	Open Manhole	600
2.001	6.420	60.0	S11	94.558	94.058	0.350	Open Manhole	1200
3.000	4.756	198.2	Dr26	94.600	94.166	0.134	Open Manhole	600
3.001	10.755	60.1	S11	94.558	93.987	0.421	Open Manhole	1200
1.003	19.978	199.8	S10	94.783	93.386	1.172	Open Manhole	1200
4.000	21.334	199.4	S16	94.954	92.199	2.530	Open Manhole	1200
4.001	7.901	197.5	S10	94.783	92.159	2.399	Open Manhole	1200
5.000	6.877	202.3	Dr28	95.050	94.606	0.144	Open Manhole	600
5.001	10.286	60.2	S10	94.783	94.435	0.198	Open Manhole	1200
6.000	2.500	192.3	Dr30	95.075	94.652	0.123	Open Manhole	600
6.001	7.206	56.7	S10	94.783	94.526	0.107	Open Manhole	1200
1.004	22.873	200.6	SS.1	94.660	92.046	2.389	Open Manhole	1200
7.000	5.100	196.2	Dr32	94.850	94.414	0.136	Open Manhole	600
7.001	5.293	60.1	SS.1	94.660	94.326	0.184	Open Manhole	1200
8.000	22.568	199.7	CP19	94.850	94.160	0.020	Open Manhole	600

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PIPELINE SCHEDULES for Storm

Upstream Manhole

PN	Byd Sect	Diam (mm)	MB Name	C.Level (m)	I.Level (m)	D.Depth (m)	MB Connection	MR DIAM., I*W (mm)
8.001	o	150	CP19	94.850	94.160	0.540	Open Manhole	600
1.005	o	225	S9.1	94.660	92.046	2.389	Open Manhole	1200
1.006	o	225	S9	94.596	91.998	2.373	Open Manhole	1200
9.000	- -		Dr33	94.600	94.190	0.110	Open Manhole	600
9.001	o	150	Dr34	94.600	94.164	0.286	Open Manhole	600
10.000	- -		CP23	94.600	94.000	0.000	Open Manhole	600
10.001	o	225	CP22	94.600	93.886	0.489	Open Manhole	600
1.007	o	225	S8.1	94.378	91.920	2.233	Open Manhole	1200
11.000	- -		Dr35	94.450	94.040	0.110	Open Manhole	600
11.001	o	150	Dr36	94.450	94.014	0.286	Open Manhole	600
1.008	o	225	S8	94.361	91.869	2.267	Open Manhole	1200
12.000	- -		CP26	94.450	93.850	0.000	Open Manhole	600
12.001	o	150	CP25	94.450	93.722	0.578	Open Manhole	600
12.002	o	150	AJ24	94.400	93.610	0.640	Open Manhole	600
1.009	o	225	S7	94.286	91.824	2.237	Open Manhole	1200
13.000	- -		Dr37	94.300	93.890	0.110	Open Manhole	600
13.001	o	150	Dr38	94.300	93.860	0.290	Open Manhole	600

Downstream Manhole

PN	Length (m)	Slope (1:X)	MB Name	C.Level (m)	I.Level (m)	D.Depth (m)	MB Connection	MR DIAM., I*W (mm)
8.001	26.567	199.8	S9.1	94.660	94.027	0.483	Open Manhole	1200
1.005	9.606	200.2	S9	94.596	91.998	2.373	Open Manhole	1200
1.006	15.627	200.3	S8.1	94.378	91.920	2.233	Open Manhole	1200
9.000	5.200	200.0	Dr34	94.600	94.164	0.136	Open Manhole	600
9.001	5.800	59.8	S8.1	94.378	94.067	0.161	Open Manhole	1200
10.000	22.836	200.3	CP22	94.600	93.886	0.114	Open Manhole	600
10.001	27.438	200.3	S8.1	94.378	93.749	0.404	Open Manhole	1200
1.007	10.126	198.5	S8	94.361	91.869	2.267	Open Manhole	1200
11.000	5.200	200.0	Dr36	94.450	94.014	0.136	Open Manhole	600
11.001	6.000	60.0	S8	94.361	93.914	0.297	Open Manhole	1200
1.008	9.068	201.5	S7	94.286	91.824	2.237	Open Manhole	1200
12.000	25.661	200.5	CP25	94.450	93.722	0.128	Open Manhole	600
12.001	22.481	200.7	AJ24	94.400	93.610	0.640	Open Manhole	600
12.002	18.920	199.2	S7	94.286	93.515	0.621	Open Manhole	1200
1.009	6.046	201.5	S6	94.347	91.794	2.328	Open Manhole	1200
13.000	6.000	200.0	Dr38	94.300	93.860	0.140	Open Manhole	600
13.001	16.200	60.0	S6	94.347	93.590	0.607	Open Manhole	1200

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PIPELINE SCHEDULES for Storm

Upstream Manhole

PN	Hyd Sect	Diam (mm)	MR Name	C.Level (m)	I.Level (m)	D.Depth (m)	MR Connection	MR DIAM., I*W (mm)
1.010	o	225	S6	94.347	91.794	2.328	Open Manhole	1200
14.000	- _ -		CP30	94.150	93.550	0.000	Open Manhole	600
14.001	o	150	CP29	94.150	93.492	0.508	Open Manhole	600
14.002	o	150	CP29.1	94.100	93.407	0.543	Open Manhole	600
1.011	o	225	S5.1	94.074	91.679	2.170	Open Manhole	1200
1.012	o	225	S5	93.988	91.643	2.120	Open Manhole	1200
15.000	- -		Dr01	94.375	93.965	0.110	Open Manhole	600
15.001	o	150	Dr02	94.375	93.939	0.286	Open Manhole	600
16.000	- -		Dr03	94.375	93.965	0.110	Open Manhole	600
16.001	o	150	Dr04	94.375	93.934	0.291	Open Manhole	600
15.002	o	225	S46	94.437	92.663	1.549	Open Manhole	1200
17.000	- -		Dr05	94.150	93.740	0.110	Open Manhole	600
17.001	o	150	Dr06	94.150	93.555	0.445	Open Manhole	600
18.000	- _ -		CP50	94.375	93.702	0.124	Open Manhole	600
18.001	- _ -		CP49	94.150	93.550	0.000	Open Manhole	600
18.002	o	150	CP48	94.150	93.435	0.565	Open Manhole	600
18.003	o	150	AJ47	94.280	93.351	0.779	Open Manhole	600
15.003	o	225	S45	94.093	92.533	1.335	Open Manhole	1200

Downstream Manhole

PN	Length (m)	Slope (1:X)	MR Name	C.Level (m)	I.Level (m)	D.Depth (m)	MR Connection	MR DIAM., I*W (mm)
1.010	22.964	199.7	S5.1	94.074	91.679	2.170	Open Manhole	1200
14.000	11.533	198.8	CP29	94.150	93.492	0.058	Open Manhole	600
14.001	16.960	199.5	CP29.1	94.100	93.407	0.543	Open Manhole	600
14.002	13.994	199.9	S5.1	94.074	93.337	0.587	Open Manhole	1200
1.011	7.237	201.0	S5	93.988	91.643	2.120	Open Manhole	1200
1.012	7.200	200.0	S4	94.059	91.607	2.227	Open Manhole	1200
15.000	5.232	201.2	Dr02	94.375	93.939	0.136	Open Manhole	600
15.001	5.400	60.0	S46	94.437	93.849	0.438	Open Manhole	1200
16.000	6.221	200.7	Dr04	94.375	93.934	0.141	Open Manhole	600
16.001	9.359	60.0	S46	94.437	93.778	0.509	Open Manhole	1200
15.002	25.983	199.9	S45	94.093	92.533	1.335	Open Manhole	1200
17.000	6.952	198.6	Dr06	94.150	93.705	0.145	Open Manhole	600
17.001	8.258	59.8	S45	94.093	93.417	0.526	Open Manhole	1200
18.000	20.311	199.1	CP49	94.150	93.600	0.001	Open Manhole	600
18.001	12.993	199.9	CP48	94.150	93.485	0.065	Open Manhole	600
18.002	16.730	199.2	AJ47	94.280	93.351	0.779	Open Manhole	600
18.003	15.344	199.3	S45	94.093	93.274	0.669	Open Manhole	1200
15.003	20.391	199.9	S44	93.831	92.431	1.175	Open Manhole	1200



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PIPELINE SCHEDULES for Storm

Upstream Manhole

PN	Byd Sect	Diam (mm)	MR Name	C.Level (m)	I.Level (m)	D.Depth (m)	MR Connection	MB DIAM., L*W (mm)
19.000	- _ -		CP55	94.600	94.000	0.000	Open Manhole	600
19.001	- _ -		CP54	94.375	93.619	0.000	Open Manhole	600
19.002	o	150	CP53	94.375	93.267	0.958	Open Manhole	600
19.003	o	150	CP52	94.375	92.977	1.248	Open Manhole	600
19.004	-\ /-		J54	94.375	92.912	0.348	Open Manhole	1200
19.005	o	225	J55	94.000	92.485	1.290	Open Manhole	1200
19.006	o	225	S51	93.834	92.458	1.151	Open Manhole	1200
20.000	- _ -		Dr07	93.895	93.485	0.110	Open Manhole	600
20.001	o	150	Dr08	93.895	93.452	0.293	Open Manhole	600
15.004	o	225	S44	93.831	92.431	1.175	Open Manhole	1200
15.005	o	225	S43	93.667	92.171	1.271	Open Manhole	1200
15.006	o	225	S42	93.712	92.110	1.377	Open Manhole	1200
21.000	- _ -		CP59	93.895	93.295	0.000	Open Manhole	600
21.001	- _ -		CP58	93.895	93.164	0.000	Open Manhole	600
21.002	- _ -		CP57	93.895	93.073	0.022	Open Manhole	600
21.003	o	150	CP56	93.895	92.562	1.183	Open Manhole	600
15.007	o	225	S41	93.672	92.060	1.367	Open Manhole	1200
15.008	o	225	S40	93.633	92.040	1.368	Open Manhole	1200
22.000	- _ -		X12	93.840	93.380	0.176	Open Manhole	600
22.001	o	225	X13	93.630	93.346	0.059	Open Manhole	600
22.002	o	225	S62	93.641	92.148	1.268	Open Manhole	600

Downstream Manhole

PN	Length (m)	Slope (1:X)	MR Name	C.Level (m)	I.Level (m)	D.Depth (m)	MR Connection	MB DIAM., L*W (mm)
19.000	17.678	77.9	CP54	94.375	93.773	0.002	Open Manhole	600
19.001	12.792	99.9	CP53	94.375	93.491	0.128	Open Manhole	600
19.002	17.417	60.1	CP52	94.375	92.977	1.248	Open Manhole	600
19.003	6.458	99.4	J54	94.375	92.912	1.313	Open Manhole	1200
19.004	13.276	491.7	J55	94.000	92.885	0.000	Open Manhole	1200
19.005	5.446	201.7	S51	93.834	92.458	1.151	Open Manhole	1200
19.006	5.379	199.2	S44	93.831	92.431	1.175	Open Manhole	1200
20.000	6.570	199.1	Dr08	93.895	93.452	0.143	Open Manhole	600
20.001	12.739	60.1	S44	93.831	93.240	0.441	Open Manhole	1200
15.004	51.961	199.9	S43	93.667	92.171	1.271	Open Manhole	1200
15.005	12.244	200.7	S42	93.712	92.110	1.377	Open Manhole	1200
15.006	9.948	199.0	S41	93.672	92.060	1.367	Open Manhole	1200
21.000	16.200	200.0	CP58	93.895	93.214	0.081	Open Manhole	600
21.001	8.203	200.1	CP57	93.895	93.123	0.041	Open Manhole	600
21.002	15.243	200.6	CP56	93.895	92.997	0.098	Open Manhole	600
21.003	7.611	200.3	S41	93.672	92.524	0.998	Open Manhole	1200
15.007	4.097	204.9	S40	93.633	92.040	1.368	Open Manhole	1200
15.008	5.743	198.0	S39	93.872	92.011	1.636	Open Manhole	1200
22.000	17.000	500.0	X13	93.630	93.346	0.000	Open Manhole	600
22.001	6.700	197.1	S62	93.641	93.312	0.104	Open Manhole	600
22.002	10.592	199.8	S61	93.820	92.095	1.500	Open Manhole	1200

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PIPELINE SCHEDULES for Storm

Upstream Manhole

PN	Ryd Sect	Diam (mm)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
23.000	-[i]-		Dr09	94.150	93.740	0.110	Open Manhole	600
23.001	o	150	Dr10	94.150	93.713	0.287	Open Manhole	600
23.002	o	225	S64	94.009	92.236	1.548	Open Manhole	600
24.000	-[i]-		Dr11	93.910	93.500	0.110	Open Manhole	600
24.001	o	150	Dr12	93.910	93.466	0.294	Open Manhole	600
24.002	o	225	S65	93.911	92.230	1.456	Open Manhole	600
25.000	o	100	XX1	93.600	92.700	0.800	Open Manhole	600
23.003	o	225	S63	93.600	92.188	1.187	Open Manhole	1200
26.000	o	225	AJDP01	93.750	93.340	0.185	Open Manhole	1200
26.001	o	225	AJDP02	93.750	93.226	0.299	Open Manhole	1200
26.002	o	225	AJDP03	93.750	93.114	0.411	Open Manhole	1200
27.000	o	225	AJDP05	93.750	93.340	0.185	Open Manhole	1200
27.001	o	225	AJDP06	93.750	93.250	0.275	Open Manhole	1200
26.003	o	225	AJDP04	93.750	93.037	0.488	Open Manhole	1200
22.003	o	225	S61	93.820	92.095	1.500	Open Manhole	1200
22.004	-[i]		S61.1	93.775	92.078	0.335	Open Manhole	1200
22.005	o	225	S60	93.872	92.030	1.617	Open Manhole	1200
15.009	o	225	S39	93.872	92.011	1.636	Open Manhole	1200
15.010	o	225	S38	93.881	91.961	1.695	Open Manhole	1200

Downstream Manhole

PN	Length (m)	Slope (1:X)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
23.000	5.400	200.0	Dr10	94.150	93.713	0.137	Open Manhole	600
23.001	9.512	59.8	S64	94.009	93.554	0.305	Open Manhole	600
23.002	9.647	201.0	S63	93.600	92.188	1.187	Open Manhole	1200
24.000	6.750	198.5	Dr12	93.910	93.466	0.144	Open Manhole	600
24.001	8.660	60.1	S65	93.911	93.322	0.439	Open Manhole	600
24.002	8.459	201.4	S63	93.600	92.188	1.187	Open Manhole	1200
25.000	12.500	500.0	S63	93.600	92.675	0.825	Open Manhole	1200
23.003	18.547	199.4	S61	93.820	92.095	1.500	Open Manhole	1200
26.000	22.748	199.5	AJDP02	93.750	93.226	0.299	Open Manhole	1200
26.001	22.445	200.4	AJDP03	93.750	93.114	0.411	Open Manhole	1200
26.002	15.387	199.8	AJDP04	93.750	93.037	0.488	Open Manhole	1200
27.000	18.014	200.2	AJDP06	93.750	93.250	0.275	Open Manhole	1200
27.001	14.719	69.1	AJDP04	93.750	93.037	0.488	Open Manhole	1200
26.003	12.980	199.7	S61	93.820	92.972	0.623	Open Manhole	1200
22.003	3.500	194.4	S61.1	93.775	92.077	1.473	Open Manhole	1200
22.004	28.142	586.3	S60	93.872	92.030	0.480	Open Manhole	1200
22.005	3.605	200.3	S39	93.872	92.012	1.635	Open Manhole	1200
15.009	9.910	198.2	S38	93.881	91.961	1.695	Open Manhole	1200
15.010	4.285	204.0	S37	93.910	91.940	1.745	Open Manhole	1200

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PIPELINE SCHEDULES for Storm

Upstream Manhole

PN	Hyd Sect	Diam (mm)	MR Name	C.Level (m)	I.Level (m)	D.Depth (m)	MR Connection	MR DIAM., L*W (mm)
15.011	o	225	S37	93.910	91.940	1.745	Open Manhole	1200
15.012	o	225	S36	93.809	91.875	1.709	Open Manhole	1200
28.000	- _ -		CP68	94.150	93.550	0.000	Open Manhole	600
28.001	- _ -		CP67	94.150	93.310	0.000	Open Manhole	600
28.002	o	150	CP66	93.910	92.472	1.288	Open Manhole	600
15.013	o	225	S35	93.757	91.845	1.687	Open Manhole	1200
29.000	- -		Dr13	94.000	93.590	0.110	Open Manhole	600
29.001	o	150	Dr14	94.000	93.565	0.285	Open Manhole	600
30.000	- _ -		CP71	94.335	93.735	0.000	Open Manhole	600
30.001	- _ -		CP70	94.335	93.285	0.365	Open Manhole	600
30.002	o	150	CP69	93.885	92.517	1.218	Open Manhole	600
31.000	- _ -		CP75	94.050	93.424	0.000	Open Manhole	600
32.000	- _ -		CP76	94.000	93.400	0.000	Open Manhole	600
31.001	o	150	CP74	94.000	93.272	0.578	Open Manhole	600
31.002	o	150	AJ73	94.130	93.187	0.793	Open Manhole	600
15.014	o	225	S34	93.801	91.813	1.763	Open Manhole	1200
33.000	- -		Dr15	94.050	93.640	0.110	Open Manhole	600
33.001	o	150	Dr16	94.050	93.612	0.288	Open Manhole	600

Downstream Manhole

PN	Length (m)	Slope (1:X)	MR Name	C.Level (m)	I.Level (m)	D.Depth (m)	MR Connection	MR DIAM., L*W (mm)
15.011	12.967	199.5	S36	93.809	91.875	1.709	Open Manhole	1200
15.012	6.043	201.4	S35	93.757	91.845	1.687	Open Manhole	1200
28.000	22.392	199.9	CP67	94.150	93.436	0.112	Open Manhole	600
28.001	13.233	200.5	CP66	93.910	93.244	-0.174	Open Manhole	600
28.002	12.869	201.1	S35	93.757	92.408	1.199	Open Manhole	1200
15.013	6.306	197.1	S34	93.801	91.813	1.763	Open Manhole	1200
29.000	5.000	200.0	Dr14	94.000	93.565	0.135	Open Manhole	600
29.001	11.100	60.0	S34	93.801	93.380	0.271	Open Manhole	1200
30.000	17.857	200.6	CP70	94.335	93.646	0.089	Open Manhole	600
30.001	17.475	200.9	CP69	93.885	93.198	0.002	Open Manhole	600
30.002	13.076	201.2	S34	93.801	92.452	1.199	Open Manhole	1200
31.000	20.489	200.9	CP74	94.000	93.322	0.052	Open Manhole	600
32.000	15.560	199.5	CP74	94.000	93.322	0.078	Open Manhole	600
31.001	16.943	199.3	AJ73	94.130	93.187	0.793	Open Manhole	600
31.002	13.606	200.1	S34	93.801	93.119	0.532	Open Manhole	1200
15.014	21.973	199.8	S33	93.922	91.703	1.994	Open Manhole	1200
33.000	5.600	200.0	Dr16	94.050	93.612	0.138	Open Manhole	600
33.001	10.500	60.0	S33	93.922	93.437	0.335	Open Manhole	1200

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PIPELINE SCHEDULES for Storm

Upstream Manhole

PN	Byd Sect	Diam (mm)	ME Name	C.Level (m)	I.Level (m)	D.Depth (m)	ME Connection	ME DIAM., L*W (mm)
15.015	o	225	S33	93.922	91.703	1.994	Open Manhole	1200
34.000	- 1 -		Dr17	94.335	93.925	0.110	Open Manhole	600
34.001	o	150	Dr18	94.335	93.899	0.286	Open Manhole	600
34.002	o	225	S77	94.225	93.135	0.865	Open Manhole	1200
35.000	- 1 -		Dr19	93.885	93.475	0.110	Open Manhole	600
35.001	o	150	Dr20	93.885	93.200	0.535	Open Manhole	600
36.000	- 1 -		CP81	94.150	93.550	0.000	Open Manhole	600
36.001	o	150	CP80	94.150	93.406	0.594	Open Manhole	600
36.002	o	150	AP79	94.280	93.320	0.810	Open Manhole	600
15.016	o	225	S32	93.898	91.671	2.002	Open Manhole	1200
37.000	- 1 -		Dr21	94.150	93.740	0.110	Open Manhole	1200
37.001	o	150	Dr22	94.150	93.711	0.289	Open Manhole	1200
15.017	o	225	S31	93.900	91.640	2.035	Open Manhole	1200
1.013	o	225	S4	94.059	91.607	2.227	Open Manhole	1200
1.014	-[1]		S3	94.078	91.601	0.730	Open Manhole	1200
1.015	o	225	S2	93.415	91.447	1.743	Open Manhole	1200

Downstream Manhole

PN	Length (m)	Slope (1:X)	ME Name	C.Level (m)	I.Level (m)	D.Depth (m)	ME Connection	ME DIAM., L*W (mm)
15.015	6.467	202.1	S32	93.898	91.671	2.002	Open Manhole	1200
34.000	5.200	200.0	Dr18	94.335	93.899	0.136	Open Manhole	600
34.001	5.900	60.2	S77	94.225	93.801	0.274	Open Manhole	1200
34.002	44.743	199.7	S32	93.898	92.911	0.762	Open Manhole	1200
35.000	5.200	208.0	Dr20	93.885	93.450	0.135	Open Manhole	600
35.001	6.500	60.2	S32	93.898	93.092	0.656	Open Manhole	1200
36.000	18.820	200.2	CP80	94.150	93.456	0.094	Open Manhole	600
36.001	17.126	199.1	AP79	94.280	93.320	0.810	Open Manhole	600
36.002	13.516	198.8	S32	93.898	93.252	0.496	Open Manhole	1200
15.016	6.287	202.8	S31	93.900	91.640	2.035	Open Manhole	1200
37.000	5.740	197.9	Dr22	94.150	93.711	0.139	Open Manhole	1200
37.001	11.632	200.6	S31	93.900	93.653	0.097	Open Manhole	1200
15.017	7.021	200.6	S4	94.059	91.605	2.229	Open Manhole	1200
1.013	3.017	502.8	S3	94.078	91.601	2.252	Open Manhole	1200
1.014	24.822	496.4	S2	93.415	91.551	0.117	Open Manhole	1200
1.015	4.410	200.5		91.916	91.425	0.266	Open Manhole	1200

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Free Flowing Outfall Details for Storm

Outfall Pipe Number	Outfall Name	C. Level (m)	I. Level (m)	Min I. Level (mm)	D,L (mm)	W
1.015		91.916	91.425	0.000	1200	0

Simulation Criteria for Storm

Volumetric Runoff Coeff 0.750 Hot Start Level (mm) 0 Additional Flow - % of Total Flow 0.000 Flow per Person per Day (l/per/day) 0.000
 Areal Reduction Factor 1.000 Manhole Headloss Coeff (Global) 0.500 MADD Factor * 10m³/ha Storage 2.000 Run Time (mins) 60
 Hot Start (mins) 0 Foul Sewage per hectare (l/s) 0.000 Inlet Coefficient 0.800 Output Interval (mins) 1

Number of Input Hydrographs 0 Number of Online Controls 22 Number of Offline Controls 0 Number of Storage Structures 42 Number of Time/Area Diagrams 61 Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model FSR Region Scotland and Ireland Ratio R 0.278 Cv (Summer) 0.750 Storm Duration (mins) 30
 Return Period (years) 2 M5-60 (mm) 16.300 Profile Type Summer Cv (Winter) 0.840

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Online Controls for Storm

Orifice Manhole: Dr24, DS/PN: 2.001, Volume (m³): 7.7
 Diameter (m) 0.030 Discharge Coefficient 0.600 Invert Level (m) 94.165

Orifice Manhole: Dr26, DS/PN: 3.001, Volume (m³): 9.2
 Diameter (m) 0.030 Discharge Coefficient 0.600 Invert Level (m) 94.166

Orifice Manhole: Dr28, DS/PN: 5.001, Volume (m³): 7.5
 Diameter (m) 0.030 Discharge Coefficient 0.600 Invert Level (m) 94.606

Orifice Manhole: Dr30, DS/PN: 6.001, Volume (m³): 2.4
 Diameter (m) 0.030 Discharge Coefficient 0.600 Invert Level (m) 94.653

Orifice Manhole: Dr32, DS/PN: 7.001, Volume (m³): 7.7
 Diameter (m) 0.030 Discharge Coefficient 0.600 Invert Level (m) 94.414

Orifice Manhole: Dr34, DS/PN: 9.001, Volume (m³): 10.0
 Diameter (m) 0.030 Discharge Coefficient 0.600 Invert Level (m) 94.164

Orifice Manhole: Dr36, DS/PN: 11.001, Volume (m³): 11.3
 Diameter (m) 0.030 Discharge Coefficient 0.600 Invert Level (m) 94.014

Orifice Manhole: Dr38, DS/PN: 13.001, Volume (m³): 6.1
 Diameter (m) 0.030 Discharge Coefficient 0.600 Invert Level (m) 93.860

Orifice Manhole: Dr02, DS/PN: 15.001, Volume (m³): 5.0
 Diameter (m) 0.030 Discharge Coefficient 0.600 Invert Level (m) 93.939

Orifice Manhole: Dr04, DS/PN: 16.001, Volume (m³): 9.7
 Diameter (m) 0.030 Discharge Coefficient 0.600 Invert Level (m) 93.934

Orifice Manhole: Dr06, DS/PN: 17.001, Volume (m³): 7.2
 Diameter (m) 0.030 Discharge Coefficient 0.600 Invert Level (m) 93.555

Orifice Manhole: Dr08, DS/PN: 20.001, Volume (m³): 11.6
 Diameter (m) 0.030 Discharge Coefficient 0.600 Invert Level (m) 93.452

Orifice Manhole: X13, DS/PN: 22.001, Volume (m³): 33.9
 Diameter (m) 0.030 Discharge Coefficient 0.600 Invert Level (m) 93.346

Orifice Manhole: Dr10, DS/PN: 23.001, Volume (m³): 10.5
 Diameter (m) 0.030 Discharge Coefficient 0.600 Invert Level (m) 93.713

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Orifice Manhole: Dr12, DS/PN: 24.001, Volume (m³): 10.6

Diameter (m) 0.030 Discharge Coefficient 0.600 Invert Level (m) 93.466

Hydro-Brake® Optimum Manhole: S60, DS/PN: 22.005, Volume (m³): 131.9

Unit Reference	MD-SHE-0050-9000-0550-9000	Flush-Flow™	Calculated	Sump Available	Yes	Minimum Outlet Pipe Diameter (mm)	75
Design Head (m)	0.550	Objective	Minimise upstream storage	Diameter (mm)	50	Suggested Manhole Diameter (mm)	1200
Design Flow (l/s)	0.9	Application	Surface	Invert Level (m)	92.030		

Control Points	Head (m)	Flow (l/s)	Control Points	Head (m)	Flow (l/s)	Control Points	Head (m)	Flow (l/s)	Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	0.550	0.9	Flush-Flow™	0.167	0.9	Kick-Flow®	0.355	0.7	Mean Flow over Head Range	-	0.8

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	0.9	0.400	0.8	0.800	1.1	1.400	1.4	2.000	1.6	2.600	1.8	4.000	2.2	5.500	2.6	7.000	2.9	8.500	3.2
0.200	0.9	0.500	0.9	1.000	1.2	1.600	1.4	2.200	1.7	3.000	1.9	4.500	2.3	6.000	2.7	7.500	3.0	9.000	3.3
0.300	0.8	0.600	0.9	1.200	1.3	1.800	1.5	2.400	1.7	3.500	2.1	5.000	2.4	6.500	2.8	8.000	3.1	9.500	3.3

Orifice Manhole: Dr14, DS/PN: 29.001, Volume (m³): 6.1

Diameter (m) 0.030 Discharge Coefficient 0.600 Invert Level (m) 93.565

Orifice Manhole: Dr16, DS/PN: 33.001, Volume (m³): 8.9

Diameter (m) 0.030 Discharge Coefficient 0.600 Invert Level (m) 93.612

Orifice Manhole: Dr18, DS/PN: 34.001, Volume (m³): 7.9

Diameter (m) 0.030 Discharge Coefficient 0.600 Invert Level (m) 93.899

Orifice Manhole: Dr20, DS/PN: 35.001, Volume (m³): 11.0

Diameter (m) 0.030 Discharge Coefficient 0.600 Invert Level (m) 93.200

Orifice Manhole: Dr22, DS/PN: 37.001, Volume (m³): 7.9

Diameter (m) 0.030 Discharge Coefficient 0.600 Invert Level (m) 93.711

Hydro-Brake® Optimum Manhole: S2, DS/PN: 1.015, Volume (m³): 547.6

Unit Reference	MD-SHE-0101-5400-1550-5400	Flush-Flow™	Calculated	Sump Available	Yes	Minimum Outlet Pipe Diameter (mm)	150
Design Head (m)	1.550	Objective	Minimise upstream storage	Diameter (mm)	101	Suggested Manhole Diameter (mm)	1200
Design Flow (l/s)	5.4	Application	Surface	Invert Level (m)	91.447		

Control Points	Head (m)	Flow (l/s)	Control Points	Head (m)	Flow (l/s)	Control Points	Head (m)	Flow (l/s)	Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	1.550	5.4	Flush-Flow™	0.440	5.3	Kick-Flow®	0.903	4.2	Mean Flow over Head Range	-	4.7

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	3.4	0.400	5.3	0.800	4.7	1.400	5.1	2.000	6.1	2.600	6.9	4.000	8.4	5.500	9.8	7.000	11.0	8.500	12.1
0.200	4.8	0.500	5.3	1.000	4.4	1.600	5.5	2.200	6.4	3.000	7.4	4.500	8.9	6.000	10.2	7.500	11.4	9.000	12.4
0.300	5.1	0.600	5.2	1.200	4.8	1.800	5.8	2.400	6.6	3.500	7.9	5.000	9.4	6.500	10.6	8.000	11.7	9.500	12.7

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Storage Structures for Storm

Infiltration Trench Pipe: 1.000

Manning's N 0.075 Safety Factor 2.0 Trench Width (m) 0.6 Cap Volume Depth (m) 0.800
 Infiltration Coefficient Base (m/hr) 0.01000 Porosity 0.30 Trench Length (m) 12.7 Cap Infiltration Depth (m) 0.800
 Infiltration Coefficient Side (m/hr) 0.01000 Invert Level (m) 94.000 Slope (1:X) 198.5

Porous Car Park Pipe: 2.000

Manning's N 0.075 Max Percolation (l/s) 26.7 Invert Level (m) 94.190 Slope (1:X) 199.0 Membrane Depth (mm) 110
 Infiltration Coefficient Base (m/hr) 0.00000 Safety Factor 2.0 Width (m) 19.3 Depression Storage (mm) 5
 Membrane Percolation (mm/hr) 1000 Porosity 0.30 Length (m) 5.0 Evaporation (mm/day) 3

Porous Car Park Pipe: 3.000

Manning's N 0.075 Max Percolation (l/s) 27.6 Invert Level (m) 94.190 Slope (1:X) 198.2 Membrane Depth (mm) 110
 Infiltration Coefficient Base (m/hr) 0.00000 Safety Factor 2.0 Width (m) 20.9 Depression Storage (mm) 5
 Membrane Percolation (mm/hr) 1000 Porosity 0.35 Length (m) 4.8 Evaporation (mm/day) 3

Porous Car Park Pipe: 5.000

Manning's N 0.075 Max Percolation (l/s) 25.0 Invert Level (m) 94.640 Slope (1:X) 202.3 Membrane Depth (mm) 110
 Infiltration Coefficient Base (m/hr) 0.00000 Safety Factor 2.0 Width (m) 13.1 Depression Storage (mm) 5
 Membrane Percolation (mm/hr) 1000 Porosity 0.30 Length (m) 6.9 Evaporation (mm/day) 3

Porous Car Park Pipe: 6.000

Manning's N 0.075 Max Percolation (l/s) 9.2 Invert Level (m) 94.665 Slope (1:X) 192.3 Membrane Depth (mm) 110
 Infiltration Coefficient Base (m/hr) 0.00000 Safety Factor 2.0 Width (m) 13.2 Depression Storage (mm) 5
 Membrane Percolation (mm/hr) 1000 Porosity 0.30 Length (m) 2.5 Evaporation (mm/day) 3

Porous Car Park Pipe: 7.000

Manning's N 0.075 Max Percolation (l/s) 26.6 Invert Level (m) 94.440 Slope (1:X) 196.2 Membrane Depth (mm) 110
 Infiltration Coefficient Base (m/hr) 0.00000 Safety Factor 2.0 Width (m) 18.8 Depression Storage (mm) 5
 Membrane Percolation (mm/hr) 1000 Porosity 0.30 Length (m) 5.1 Evaporation (mm/day) 3

Infiltration Trench Pipe: 8.000

Manning's N 0.075 Safety Factor 2.0 Trench Width (m) 0.6 Cap Volume Depth (m) 0.670
 Infiltration Coefficient Base (m/hr) 0.01000 Porosity 0.35 Trench Length (m) 22.6 Cap Infiltration Depth (m) 0.670
 Infiltration Coefficient Side (m/hr) 0.01000 Invert Level (m) 94.273 Slope (1:X) 199.7

Porous Car Park Pipe: 9.000

Manning's N 0.075 Max Percolation (l/s) 34.4 Invert Level (m) 94.190 Slope (1:X) 200.0 Membrane Depth (mm) 110
 Infiltration Coefficient Base (m/hr) 0.00000 Safety Factor 2.0 Width (m) 23.8 Depression Storage (mm) 5
 Membrane Percolation (mm/hr) 1000 Porosity 0.30 Length (m) 5.2 Evaporation (mm/day) 3

Infiltration Trench Pipe: 10.000

Manning's N 0.075 Safety Factor 2.0 Trench Width (m) 0.6 Cap Volume Depth (m) 0.800
 Infiltration Coefficient Base (m/hr) 0.01000 Porosity 0.35 Trench Length (m) 22.8 Cap Infiltration Depth (m) 0.800
 Infiltration Coefficient Side (m/hr) 0.01000 Invert Level (m) 94.000 Slope (1:X) 200.3

Porous Car Park Pipe: 11.000

Manning's N 0.075 Membrane Percolation (mm/hr) 1000 Safety Factor 2.0 Invert Level (m) 94.040 Length (m) 5.2
 Infiltration Coefficient Base (m/hr) 0.00000 Max Percolation (l/s) 39.1 Porosity 0.30 Width (m) 27.1 Slope (1:X) 200.0

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Porous Car Park Pipe: 11.000

Depression Storage (mm) 5 Evaporation (mm/day) 3 Membrane Depth (mm) 110

Infiltration Trench Pipe: 12.000

Manning's N 0.075 Safety Factor 2.0 Trench Width (m) 0.6 Cap Volume Depth (m) 0.800
Infiltration Coefficient Base (m/hr) 0.01000 Porosity 0.35 Trench Length (m) 25.7 Cap Infiltration Depth (m) 0.800
Infiltration Coefficient Side (m/hr) 0.01000 Invert Level (m) 93.850 Slope (1:X) 200.5

Porous Car Park Pipe: 13.000

Manning's N 0.075 Max Percolation (l/s) 20.3 Invert Level (m) 93.890 Slope (1:X) 200.0 Membrane Depth (mm) 110
Infiltration Coefficient Base (m/hr) 0.00000 Safety Factor 2.0 Width (m) 12.2 Depression Storage (mm) 5
Membrane Percolation (mm/hr) 1000 Porosity 0.30 Length (m) 6.0 Evaporation (mm/day) 3

Infiltration Trench Pipe: 14.000

Manning's N 0.075 Safety Factor 2.0 Trench Width (m) 0.6 Cap Volume Depth (m) 0.800
Infiltration Coefficient Base (m/hr) 0.01000 Porosity 0.30 Trench Length (m) 11.5 Cap Infiltration Depth (m) 0.800
Infiltration Coefficient Side (m/hr) 0.01000 Invert Level (m) 93.550 Slope (1:X) 198.8

Porous Car Park Pipe: 15.000

Manning's N 0.075 Max Percolation (l/s) 17.1 Invert Level (m) 93.965 Slope (1:X) 201.2 Membrane Depth (mm) 110
Infiltration Coefficient Base (m/hr) 0.00000 Safety Factor 2.0 Width (m) 11.8 Depression Storage (mm) 5
Membrane Percolation (mm/hr) 1000 Porosity 0.30 Length (m) 5.2 Evaporation (mm/day) 3

Porous Car Park Pipe: 16.000

Manning's N 0.075 Max Percolation (l/s) 32.8 Invert Level (m) 93.965 Slope (1:X) 200.7 Membrane Depth (mm) 110
Infiltration Coefficient Base (m/hr) 0.00000 Safety Factor 2.0 Width (m) 19.0 Depression Storage (mm) 5
Membrane Percolation (mm/hr) 1000 Porosity 0.30 Length (m) 6.2 Evaporation (mm/day) 3

Porous Car Park Pipe: 17.000

Manning's N 0.075 Max Percolation (l/s) 23.8 Invert Level (m) 93.740 Slope (1:X) 198.6 Membrane Depth (mm) 110
Infiltration Coefficient Base (m/hr) 0.00000 Safety Factor 2.0 Width (m) 12.3 Depression Storage (mm) 5
Membrane Percolation (mm/hr) 1000 Porosity 0.30 Length (m) 7.0 Evaporation (mm/day) 3

Infiltration Trench Pipe: 18.000

Manning's N 0.075 Safety Factor 2.0 Trench Width (m) 0.6 Cap Volume Depth (m) 0.549
Infiltration Coefficient Base (m/hr) 0.01000 Porosity 0.35 Trench Length (m) 20.3 Cap Infiltration Depth (m) 0.549
Infiltration Coefficient Side (m/hr) 0.01000 Invert Level (m) 93.702 Slope (1:X) 199.1

Infiltration Trench Pipe: 18.001

Manning's N 0.075 Safety Factor 2.0 Trench Width (m) 0.6 Cap Volume Depth (m) 0.600
Infiltration Coefficient Base (m/hr) 0.01000 Porosity 0.35 Trench Length (m) 13.0 Cap Infiltration Depth (m) 0.600
Infiltration Coefficient Side (m/hr) 0.01000 Invert Level (m) 93.550 Slope (1:X) 199.9

Infiltration Trench Pipe: 18.000

Manning's N 0.075 Safety Factor 2.0 Trench Width (m) 0.6 Cap Volume Depth (m) 0.800
Infiltration Coefficient Base (m/hr) 0.01000 Porosity 0.35 Trench Length (m) 17.7 Cap Infiltration Depth (m) 0.800
Infiltration Coefficient Side (m/hr) 0.01000 Invert Level (m) 94.000 Slope (1:X) 77.9

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Infiltration Trench Pipe: 19.001

Manning's N 0.075 Safety Factor 2.0 Trench Width (m) 0.6 Cap Volume Depth (m) 0.800
 Infiltration Coefficient Base (m/hr) 0.01000 Porosity 0.35 Trench Length (m) 12.8 Cap Infiltration Depth (m) 0.800
 Infiltration Coefficient Side (m/hr) 0.01000 Invert Level (m) 93.619 Slope (1:X) 99.9

Infiltration Basin Pipe: 19.004

Manning's N 0.035 Infiltration Coefficient Base (m/hr) 0.01000 Safety Factor 2.0
 Invert Level (m) 92.912 Infiltration Coefficient Side (m/hr) 0.01000 Porosity 1.00

Depth (m) Area (m²) Depth (m) Area (m²)

0.000 9.1 1.100 9.1

Porous Car Park Pipe: 20.000

Manning's N 0.075 Max Percolation (l/s) 38.9 Invert Level (m) 93.485 Slope (1:X) 199.1 Membrane Depth (mm) 110
 Infiltration Coefficient Base (m/hr) 0.00000 Safety Factor 2.0 Width (m) 21.3 Depression Storage (mm) 5
 Membrane Percolation (mm/hr) 1000 Porosity 0.30 Length (m) 6.6 Evaporation (mm/day) 3

Infiltration Trench Pipe: 21.000

Manning's N 0.075 Safety Factor 2.0 Trench Width (m) 0.6 Cap Volume Depth (m) 0.800
 Infiltration Coefficient Base (m/hr) 0.01000 Porosity 0.35 Trench Length (m) 16.2 Cap Infiltration Depth (m) 0.800
 Infiltration Coefficient Side (m/hr) 0.01000 Invert Level (m) 93.295 Slope (1:X) 200.0

Infiltration Trench Pipe: 21.001

Manning's N 0.075 Safety Factor 2.0 Trench Width (m) 0.6 Cap Volume Depth (m) 0.800
 Infiltration Coefficient Base (m/hr) 0.01000 Porosity 0.35 Trench Length (m) 8.2 Cap Infiltration Depth (m) 0.800
 Infiltration Coefficient Side (m/hr) 0.01000 Invert Level (m) 93.164 Slope (1:X) 200.1

Infiltration Trench Pipe: 21.002

Manning's N 0.075 Safety Factor 2.0 Trench Width (m) 0.6 Cap Volume Depth (m) 0.800
 Infiltration Coefficient Base (m/hr) 0.01000 Porosity 0.35 Trench Length (m) 15.2 Cap Infiltration Depth (m) 0.800
 Infiltration Coefficient Side (m/hr) 0.01000 Invert Level (m) 93.073 Slope (1:X) 200.6

Porous Car Park Pipe: 22.000

Manning's N 0.075 Max Percolation (l/s) 114.3 Invert Level (m) 93.380 Slope (1:X) 500.0 Membrane Depth (mm) 176
 Infiltration Coefficient Base (m/hr) 0.00000 Safety Factor 2.0 Width (m) 24.2 Depression Storage (mm) 5
 Membrane Percolation (mm/hr) 1000 Porosity 0.30 Length (m) 17.0 Evaporation (mm/day) 3

Porous Car Park Pipe: 23.000

Manning's N 0.075 Max Percolation (l/s) 36.0 Invert Level (m) 93.740 Slope (1:X) 200.0 Membrane Depth (mm) 110
 Infiltration Coefficient Base (m/hr) 0.00000 Safety Factor 2.0 Width (m) 24.0 Depression Storage (mm) 5
 Membrane Percolation (mm/hr) 1000 Porosity 0.30 Length (m) 5.4 Evaporation (mm/day) 3

Porous Car Park Pipe: 24.000

Manning's N 0.075 Max Percolation (l/s) 35.4 Invert Level (m) 93.500 Slope (1:X) 198.5 Membrane Depth (mm) 110
 Infiltration Coefficient Base (m/hr) 0.00000 Safety Factor 2.0 Width (m) 18.9 Depression Storage (mm) 5
 Membrane Percolation (mm/hr) 1000 Porosity 0.30 Length (m) 6.8 Evaporation (mm/day) 3

Cellular Storage Pipe: 22.004

Manning's N 0.050 Infiltration Coefficient Base (m/hr) 0.01000 Safety Factor 2.0
 Invert Level (m) 92.078 Infiltration Coefficient Side (m/hr) 0.01000 Porosity 1.00

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Cellular Storage Pipe: 22.004

Depth (m)	Area (m ²)	Inf. Area (m ²)	Depth (m)	Area (m ²)	Inf. Area (m ²)	Depth (m)	Area (m ²)	Inf. Area (m ²)	Depth (m)	Area (m ²)	Inf. Area (m ²)	Depth (m)	Area (m ²)	Inf. Area (m ²)
0.000	66.6	166.6	0.377	145.2	166.6	0.605	124.4	166.6	0.833	103.6	166.6	1.061	82.8	166.6
0.300	71.2	166.6	0.453	138.3	166.6	0.681	117.5	166.6	0.909	96.6	166.6	1.361	87.3	166.6
0.301	152.1	166.6	0.529	131.3	166.6	0.757	110.5	166.6	0.985	89.7	166.6	1.362	0.0	166.6

Infiltration Trench Pipe: 28.000

Manning's N 0.075 Safety Factor 2.0 Trench Width (m) 0.6 Cap Volume Depth (m) 0.800
 Infiltration Coefficient Base (m/hr) 0.01000 Porosity 0.35 Trench Length (m) 22.4 Cap Infiltration Depth (m) 0.800
 Infiltration Coefficient Side (m/hr) 0.01000 Invert Level (m) 93.550 Slope (1:X) 199.9

Infiltration Trench Pipe: 28.001

Manning's N 0.075 Safety Factor 2.0 Trench Width (m) 0.6 Cap Volume Depth (m) 0.650
 Infiltration Coefficient Base (m/hr) 0.01000 Porosity 0.35 Trench Length (m) 13.2 Cap Infiltration Depth (m) 0.650
 Infiltration Coefficient Side (m/hr) 0.01000 Invert Level (m) 93.310 Slope (1:X) 200.5

Porous Car Park Pipe: 29.000

Manning's N 0.075 Max Percolation (l/s) 20.8 Invert Level (m) 93.590 Slope (1:X) 200.0 Membrane Depth (mm) 110
 Infiltration Coefficient Base (m/hr) 0.00000 Safety Factor 2.0 Width (m) 15.0 Depression Storage (mm) 5
 Membrane Percolation (mm/hr) 1000 Porosity 0.30 Length (m) 5.0 Evaporation (mm/day) 3

Infiltration Trench Pipe: 30.000

Manning's N 0.075 Safety Factor 2.0 Trench Width (m) 0.6 Cap Volume Depth (m) 0.800
 Infiltration Coefficient Base (m/hr) 0.01000 Porosity 0.35 Trench Length (m) 17.9 Cap Infiltration Depth (m) 0.800
 Infiltration Coefficient Side (m/hr) 0.01000 Invert Level (m) 93.735 Slope (1:X) 200.6

Infiltration Trench Pipe: 30.001

Manning's N 0.075 Safety Factor 2.0 Trench Width (m) 0.6 Cap Volume Depth (m) 0.685
 Infiltration Coefficient Base (m/hr) 0.01000 Porosity 0.35 Trench Length (m) 17.5 Cap Infiltration Depth (m) 0.685
 Infiltration Coefficient Side (m/hr) 0.01000 Invert Level (m) 93.285 Slope (1:X) 200.9

Infiltration Trench Pipe: 31.000

Manning's N 0.075 Safety Factor 2.0 Trench Width (m) 0.6 Cap Volume Depth (m) 0.800
 Infiltration Coefficient Base (m/hr) 0.01000 Porosity 0.35 Trench Length (m) 20.5 Cap Infiltration Depth (m) 0.800
 Infiltration Coefficient Side (m/hr) 0.01000 Invert Level (m) 93.424 Slope (1:X) 200.9

Infiltration Trench Pipe: 32.000

Manning's N 0.075 Safety Factor 2.0 Trench Width (m) 0.6 Cap Volume Depth (m) 0.800
 Infiltration Coefficient Base (m/hr) 0.01000 Porosity 0.35 Trench Length (m) 15.6 Cap Infiltration Depth (m) 0.800
 Infiltration Coefficient Side (m/hr) 0.01000 Invert Level (m) 93.400 Slope (1:X) 199.5

Porous Car Park Pipe: 33.000

Manning's N 0.075 Max Percolation (l/s) 30.5 Invert Level (m) 93.640 Slope (1:X) 200.0 Membrane Depth (mm) 110
 Infiltration Coefficient Base (m/hr) 0.00000 Safety Factor 2.0 Width (m) 19.6 Depression Storage (mm) 5
 Membrane Percolation (mm/hr) 1000 Porosity 0.30 Length (m) 5.6 Evaporation (mm/day) 3

Porous Car Park Pipe: 34.000

Manning's N 0.075 Max Percolation (l/s) 27.0 Invert Level (m) 93.925 Slope (1:X) 200.0 Membrane Depth (mm) 110
 Infiltration Coefficient Base (m/hr) 0.00000 Safety Factor 2.0 Width (m) 18.7 Depression Storage (mm) 5
 Membrane Percolation (mm/hr) 1000 Porosity 0.30 Length (m) 5.2 Evaporation (mm/day) 3

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Porous Car Park Pipe: 35.000

Manning's N 0.075 Max Percolation (1/s) 37.6 Invert Level (m) 93.475 Slope (1:X) 208.0 Membrane Depth (mm) 110
 Infiltration Coefficient Base (m/hr) 0.00000 Safety Factor 2.0 Width (m) 26.0 Depression Storage (mm) 5
 Membrane Percolation (mm/hr) 1000 Porosity 0.30 Length (m) 5.2 Evaporation (mm/day) 3

Infiltration Trench Pipe: 36.000

Manning's N 0.075 Safety Factor 2.0 Trench Width (m) 0.6 Cap Volume Depth (m) 0.800
 Infiltration Coefficient Base (m/hr) 0.01000 Porosity 0.35 Trench Length (m) 18.8 Cap Infiltration Depth (m) 0.800
 Infiltration Coefficient Side (m/hr) 0.01000 Invert Level (m) 93.550 Slope (1:X) 200.2

Porous Car Park Pipe: 37.000

Manning's N 0.075 Max Percolation (1/s) 28.9 Invert Level (m) 93.740 Slope (1:X) 197.9 Membrane Depth (mm) 110
 Infiltration Coefficient Base (m/hr) 0.00000 Safety Factor 2.0 Width (m) 18.1 Depression Storage (mm) 5
 Membrane Percolation (mm/hr) 1000 Porosity 0.30 Length (m) 5.7 Evaporation (mm/day) 3

Cellular Storage Pipe: 1.014

Manning's N 0.050 Infiltration Coefficient Base (m/hr) 0.01000 Safety Factor 2.0
 Invert Level (m) 91.601 Infiltration Coefficient Side (m/hr) 0.01000 Porosity 1.00

Depth (m)	Area (m ²)	Inf. Area (m ²)	Depth (m)	Area (m ²)	Inf. Area (m ²)	Depth (m)	Area (m ²)	Inf. Area (m ²)	Depth (m)	Area (m ²)	Inf. Area (m ²)	Depth (m)	Area (m ²)	Inf. Area (m ²)
0.000	215.2	538.0	0.416	473.3	538.0	0.759	399.8	538.0	1.103	326.4	538.0	1.446	252.9	538.0
0.300	223.0	538.0	0.530	446.8	538.0	0.874	375.3	538.0	1.217	301.9	538.0	1.746	260.8	538.0
0.301	497.7	538.0	0.645	424.3	538.0	0.988	350.9	538.0	1.332	277.4	538.0	1.747	0.0	538.0

Time Area Diagram at Pipe Number 1.000 for Storm

Total Area (ha) 0.012

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)				
0	4 0.000	4	8 0.000	8	12 0.000	12	16 0.001	16	20 0.001	20	24 0.001	24	28 0.001	28	32 0.002	32	36 0.002	36	40 0.002	40	44 0.002

Time Area Diagram at Pipe Number 2.000 for Storm

Total Area (ha) 0.009

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)		
0	4 0.000	4	8 0.000	8	12 0.001	12	16 0.001	16	20 0.001	20	24 0.001	24	28 0.001	28	32 0.001	32	36 0.001	36	40 0.002

Time Area Diagram at Pipe Number 2.000 for Storm

Total Area (ha) 0.009

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)				
0	4 0.000	4	8 0.000	8	12 0.000	12	16 0.001	16	20 0.001	20	24 0.001	24	28 0.001	28	32 0.001	32	36 0.001	36	40 0.001	40	44 0.002

Time Area Diagram at Pipe Number 3.000 for Storm

Total Area (ha) 0.009

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)		
0	4 0.000	4	8 0.000	8	12 0.001	12	16 0.001	16	20 0.001	20	24 0.001	24	28 0.001	28	32 0.001	32	36 0.001	36	40 0.002

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Time Area Diagram at Pipe Number 3.000 for Storm

Total Area (ha) 0.012

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)				
From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:				
0	4 0.000	4	8 0.000	8	12 0.000	12	16 0.001	16	20 0.001	20	24 0.001	24	28 0.001	28	32 0.002	32	36 0.002	36	40 0.002	40	44 0.002

Time Area Diagram at Pipe Number 4.000 for Storm

Total Area (ha) 0.018

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)		
From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:		
0	4 0.000	4	8 0.000	8	12 0.002	12	16 0.002	16	20 0.002	20	24 0.002	24	28 0.002	28	32 0.002	32	36 0.003	36	40 0.003

Time Area Diagram at Pipe Number 4.000 for Storm

Total Area (ha) 0.008

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)				
From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:				
0	4 0.000	4	8 0.000	8	12 0.001	12	16 0.001	16	20 0.001	20	24 0.001	24	28 0.001	28	32 0.001	32	36 0.001	36	40 0.001

Time Area Diagram at Pipe Number 4.000 for Storm

Total Area (ha) 0.019

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)				
From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:				
0	4 0.000	4	8 0.000	8	12 0.002	12	16 0.002	16	20 0.002	20	24 0.002	24	28 0.002	28	32 0.002	32	36 0.002	36	40 0.002	40	44 0.003

Time Area Diagram at Pipe Number 6.000 for Storm

Total Area (ha) 0.003

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)
From:	To:	From:	To:	From:	To:	From:	To:	From:	To:
0	4 0.000	4	8 0.000	8	12 0.001	12	16 0.001	16	20 0.001

Time Area Diagram at Pipe Number 6.000 for Storm

Total Area (ha) 0.006

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)				
From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:				
0	4 0.000	4	8 0.000	8	12 0.000	12	16 0.001	16	20 0.001	20	24 0.001	24	28 0.001	28	32 0.001	32	36 0.001

Time Area Diagram at Pipe Number 7.000 for Storm

Total Area (ha) 0.009

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)		
From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:		
0	4 0.000	4	8 0.000	8	12 0.001	12	16 0.001	16	20 0.001	20	24 0.001	24	28 0.001	28	32 0.001	32	36 0.001	36	40 0.002



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Time Area Diagram at Pipe Number 12.000 for Storm

Total Area (ha) 0.015

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)				
From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:				
0	4 0.000	4	8 0.000	8	12 0.000	12	16 0.001	16	20 0.002	20	24 0.002	24	28 0.002	28	32 0.002	32	36 0.002	36	40 0.002	40	44 0.002

Time Area Diagram at Pipe Number 13.000 for Storm

Total Area (ha) 0.006

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)
From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:
0	4 0.000	4	8 0.000	8	12 0.001	12	16 0.001	16	20 0.001	20	24 0.001	24	28 0.001	28	32 0.001

Time Area Diagram at Pipe Number 13.000 for Storm

Total Area (ha) 0.015

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)				
From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:				
0	4 0.000	4	8 0.000	8	12 0.000	12	16 0.001	16	20 0.002	20	24 0.002	24	28 0.002	28	32 0.002	32	36 0.002	36	40 0.002	40	44 0.002

Time Area Diagram at Pipe Number 14.000 for Storm

Total Area (ha) 0.015

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)				
From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:				
0	4 0.000	4	8 0.000	8	12 0.000	12	16 0.001	16	20 0.002	20	24 0.002	24	28 0.002	28	32 0.002	32	36 0.002	36	40 0.002	40	44 0.002

Time Area Diagram at Pipe Number 15.000 for Storm

Total Area (ha) 0.006

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)
From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:
0	4 0.000	4	8 0.000	8	12 0.001	12	16 0.001	16	20 0.001	20	24 0.001	24	28 0.001	28	32 0.001

Time Area Diagram at Pipe Number 15.000 for Storm

Total Area (ha) 0.013

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)				
From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:				
0	4 0.000	4	8 0.000	8	12 0.000	12	16 0.001	16	20 0.000	20	24 0.002	24	28 0.002	28	32 0.002	32	36 0.002	36	40 0.002	40	44 0.002

Time Area Diagram at Pipe Number 16.000 for Storm

Total Area (ha) 0.012

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)		
From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:		
0	4 0.000	4	8 0.000	8	12 0.001	12	16 0.001	16	20 0.001	20	24 0.001	24	28 0.002	28	32 0.002	32	36 0.002	36	40 0.002

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Time Area Diagram at Pipe Number 16.000 for Storm

Total Area (ha) 0.013

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)		
From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:		
0	4 0.000	4	8 0.000	8	12 0.000	12	16 0.001	16	20 0.001	20	24 0.001	24	28 0.002	28	32 0.002	32	36 0.002	36	40 0.002	40	44 0.002

Time Area Diagram at Pipe Number 17.000 for Storm

Total Area (ha) 0.009

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)
From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:
0	4 0.000	4	8 0.000	8	12 0.001	12	16 0.001	16	20 0.001	20	24 0.001	24	28 0.001	28	32 0.001	32	36 0.001	36	40 0.002

Time Area Diagram at Pipe Number 17.000 for Storm

Total Area (ha) 0.009

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)		
From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:		
0	4 0.000	4	8 0.000	8	12 0.000	12	16 0.001	16	20 0.001	20	24 0.001	24	28 0.001	28	32 0.001	32	36 0.001	36	40 0.001	40	44 0.002

Time Area Diagram at Pipe Number 18.000 for Storm

Total Area (ha) 0.013

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)		
From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:		
0	4 0.000	4	8 0.000	8	12 0.000	12	16 0.001	16	20 0.000	20	24 0.002	24	28 0.002	28	32 0.002	32	36 0.002	36	40 0.002	40	44 0.002

Time Area Diagram at Pipe Number 18.001 for Storm

Total Area (ha) 0.009

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)		
From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:		
0	4 0.000	4	8 0.000	8	12 0.000	12	16 0.001	16	20 0.001	20	24 0.001	24	28 0.001	28	32 0.001	32	36 0.001	36	40 0.001	40	44 0.002

Time Area Diagram at Pipe Number 19.000 for Storm

Total Area (ha) 0.012

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)		
From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:		
0	4 0.000	4	8 0.000	8	12 0.000	12	16 0.001	16	20 0.001	20	24 0.001	24	28 0.001	28	32 0.002	32	36 0.002	36	40 0.002	40	44 0.002

Time Area Diagram at Pipe Number 19.001 for Storm

Total Area (ha) 0.013

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)		
From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:		
0	4 0.000	4	8 0.000	8	12 0.000	12	16 0.001	16	20 0.001	20	24 0.001	24	28 0.002	28	32 0.002	32	36 0.002	36	40 0.002	40	44 0.002

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Time Area Diagram at Pipe Number 20.000 for Storm

Total Area (ha) 0.014

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)		
From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:		
0	4 0.000	4	8 0.000	8	12 0.002	12	16 0.001	16	20 0.001	20	24 0.002	24	28 0.002	28	32 0.002	32	36 0.002	36	40 0.002

Time Area Diagram at Pipe Number 20.000 for Storm

Total Area (ha) 0.025

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)		
From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:		
0	4 0.000	4	8 0.000	8	12 0.000	12	16 0.003	16	20 0.003	20	24 0.003	24	28 0.003	28	32 0.003	32	36 0.003	36	40 0.003	40	44 0.004

Time Area Diagram at Pipe Number 21.000 for Storm

Total Area (ha) 0.025

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)		
From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:		
0	4 0.000	4	8 0.000	8	12 0.000	12	16 0.003	16	20 0.003	20	24 0.003	24	28 0.003	28	32 0.003	32	36 0.003	36	40 0.003	40	44 0.004

Time Area Diagram at Pipe Number 21.000 for Storm

Total Area (ha) 0.005

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)
From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:
0	4 0.000	4	8 0.000	8	12 0.001	12	16 0.001	16	20 0.001	20	24 0.001	24	28 0.001		

Time Area Diagram at Pipe Number 15.008 for Storm

Total Area (ha) 0.031

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)
From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:
0	4 0.000	4	8 0.000	8	12 0.003	12	16 0.004	16	20 0.004	20	24 0.004	24	28 0.004	28	32 0.004	32	36 0.004	36	40 0.004

Time Area Diagram at Pipe Number 15.008 for Storm

Total Area (ha) 0.075

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)
From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:
0	4 0.000	4	8 0.000	8	12 0.009	12	16 0.009	16	20 0.009	20	24 0.009	24	28 0.009	28	32 0.010	32	36 0.010	36	40 0.010

Time Area Diagram at Pipe Number 22.000 for Storm

Total Area (ha) 0.039

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)
From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:
0	4 0.000	4	8 0.000	8	12 0.004	12	16 0.005	16	20 0.005	20	24 0.005	24	28 0.005	28	32 0.005	32	36 0.005	36	40 0.005

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Time Area Diagram at Pipe Number 23.500 for Storm

Total Area (ha) 0.013

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)		
From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:		
0	4 0.000	4	8 0.000	8	12 0.001	12	16 0.001	16	20 0.001	20	24 0.002	24	28 0.002	28	32 0.002	32	36 0.002	36	40 0.002

Time Area Diagram at Pipe Number 23.000 for Storm

Total Area (ha) 0.013

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)		
From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:		
0	4 0.000	4	8 0.000	8	12 0.000	12	16 0.001	16	20 0.001	20	24 0.001	24	28 0.002	28	32 0.002	32	36 0.002	36	40 0.002	40	44 0.002

Time Area Diagram at Pipe Number 24.000 for Storm

Total Area (ha) 0.013

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)		
From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:		
0	4 0.000	4	8 0.000	8	12 0.001	12	16 0.001	16	20 0.001	20	24 0.002	24	28 0.002	28	32 0.002	32	36 0.002	36	40 0.002

Time Area Diagram at Pipe Number 24.000 for Storm

Total Area (ha) 0.014

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)		
From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:		
0	4 0.000	4	8 0.000	8	12 0.000	12	16 0.001	16	20 0.001	20	24 0.002	24	28 0.002	28	32 0.002	32	36 0.002	36	40 0.002	40	44 0.002

Time Area Diagram at Pipe Number 26.000 for Storm

Total Area (ha) 0.034

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)
From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:
0	4 0.000	8	12 0.000	16	20 0.003	24	28 0.003	32	36 0.003	40	44 0.004	48	52 0.004
4	8 0.000	12	16 0.003	20	24 0.003	28	32 0.003	36	40 0.004	44	48 0.004		

Time Area Diagram at Pipe Number 27.000 for Storm

Total Area (ha) 0.034

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)
From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:
0	4 0.000	8	12 0.000	16	20 0.003	24	28 0.003	32	36 0.003	40	44 0.004	48	52 0.004
4	8 0.000	12	16 0.003	20	24 0.003	28	32 0.003	36	40 0.004	44	48 0.004		

Time Area Diagram at Pipe Number 28.000 for Storm

Total Area (ha) 0.016

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)				
From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:				
0	4 0.000	4	8 0.000	8	12 0.000	12	16 0.002	16	20 0.002	20	24 0.002	24	28 0.002	28	32 0.002	32	36 0.002	36	40 0.002	40	44 0.002

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Time Area Diagram at Pipe Number 28.001 for Storm

Total Area (ha) 0.013

Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)												
0	4	0.000	4	8	0.000	8	12	0.001	12	16	0.001	16	20	0.001	20	24	0.002	24	28	0.002	28	32	0.002	32	36	0.002	36	40	0.002

Time Area Diagram at Pipe Number 29.000 for Storm

Total Area (ha) 0.008

Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)												
0	4	0.000	4	8	0.000	8	12	0.001	12	16	0.001	16	20	0.001	20	24	0.001	24	28	0.001	28	32	0.001	32	36	0.001	36	40	0.001

Time Area Diagram at Pipe Number 29.000 for Storm

Total Area (ha) 0.011

Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)			
0	4	0.000	4	8	0.000	8	12	0.000	12	16	0.001	16	20	0.001	20	24	0.001	24	28	0.001	28	32	0.001	32	36	0.002	36	40	0.002	40	44	0.002

Time Area Diagram at Pipe Number 30.000 for Storm

Total Area (ha) 0.013

Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)												
0	4	0.000	4	8	0.000	8	12	0.001	12	16	0.001	16	20	0.001	20	24	0.002	24	28	0.002	28	32	0.002	32	36	0.002	36	40	0.002

Time Area Diagram at Pipe Number 30.001 for Storm

Total Area (ha) 0.017

Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)									
4	8	0.000	8	12	0.000	12	16	0.002	16	20	0.002	20	24	0.002	24	28	0.002	28	32	0.002	32	36	0.002	36	40	0.003

Time Area Diagram at Pipe Number 32.000 for Storm

Total Area (ha) 0.011

Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)												
0	4	0.000	4	8	0.000	8	12	0.001	12	16	0.001	16	20	0.001	20	24	0.001	24	28	0.001	28	32	0.002	32	36	0.002	36	40	0.002

Time Area Diagram at Pipe Number 33.000 for Storm

Total Area (ha) 0.011

Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)	Time (mins) From:	Time (mins) To:	Area (ha)															
0	4	0.000	4	8	0.000	8	12	0.001	12	16	0.001	16	20	0.001	20	24	0.001	24	28	0.001	28	32	0.002	32	36	0.001	36	40	0.001	40	44	0.002

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Time Area Diagram at Pipe Number 33.000 for Storm

Total Area (ha) 0.013

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)		
From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)		
0	4 0.000	4	8 0.000	8	12 0.001	12	16 0.001	16	20 0.001	20	24 0.002	24	28 0.002	28	32 0.002	32	36 0.002	36	40 0.002

Time Area Diagram at Pipe Number 34.000 for Storm

Total Area (ha) 0.013

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)		
From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)		
0	4 0.000	4	8 0.000	8	12 0.000	12	16 0.001	16	20 0.001	20	24 0.001	24	28 0.002	28	32 0.002	32	36 0.002	36	40 0.002	40	44 0.002

Time Area Diagram at Pipe Number 34.000 for Storm

Total Area (ha) 0.009

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)		
From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)		
0	4 0.000	4	8 0.000	8	12 0.001	12	16 0.001	16	20 0.001	20	24 0.001	24	28 0.001	28	32 0.001	32	36 0.001	36	40 0.002

Time Area Diagram at Pipe Number 35.000 for Storm

Total Area (ha) 0.012

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)		
From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)		
0	4 0.000	4	8 0.000	8	12 0.001	12	16 0.001	16	20 0.002	20	24 0.002	24	28 0.002	28	32 0.002	32	36 0.002

Time Area Diagram at Pipe Number 35.000 for Storm

Total Area (ha) 0.017

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)		
From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)		
0	4 0.000	4	8 0.000	8	12 0.000	12	16 0.002	16	20 0.002	20	24 0.002	24	28 0.002	28	32 0.002	32	36 0.002	36	40 0.002	40	44 0.003

Time Area Diagram at Pipe Number 36.000 for Storm

Total Area (ha) 0.014

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)		
From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)		
0	4 0.000	4	8 0.000	8	12 0.001	12	16 0.001	16	20 0.002	20	24 0.002	24	28 0.002	28	32 0.002	32	36 0.002	36	40 0.002

Time Area Diagram at Pipe Number 37.000 for Storm

Total Area (ha) 0.010

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)		
From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)	From: To:	(ha)		
0	4 0.000	4	8 0.000	8	12 0.001	12	16 0.001	16	20 0.001	20	24 0.001	24	28 0.001	28	32 0.001	32	36 0.002	36	40 0.002

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Time Area Diagram at Pipe Number 37.000 for Storm

Total Area (ha) 0.014

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)				
From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:	From:	To:				
0	4 0.000	4	8 0.000	8	12 0.000	12	16 0.001	16	20 0.001	20	24 0.002	24	28 0.002	28	32 0.002	32	36 0.002	36	40 0.002	40	44 0.002

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Volume Summary (Static)

Length Calculations based on Centre-Centre

Pipe Number	USMG Name	Manhole Volume (m³)	Pipe Volume (m³)	Storage	Total Volume (m³)
				Structure Volume (m³)	
1.000	CP15	0.170	1.372	0.000	1.541
1.001	CP14	0.202	0.299	0.000	0.501
1.002	AJ13	0.169	0.254	0.000	0.424
2.000	Dr23	0.116	8.642	0.000	8.757
2.001	Dr24	0.123	0.113	0.000	0.236
3.000	Dr25	0.116	10.437	0.000	10.553
3.001	Dr26	0.123	0.190	0.000	0.313
1.003	S11	1.212	0.794	0.000	2.007
4.000	S17	3.613	0.848	0.000	4.462
4.001	S16	3.116	0.314	0.000	3.430
5.000	Dr27	0.116	8.108	0.000	8.224
5.001	Dr28	0.126	0.182	0.000	0.307
6.000	Dr29	0.116	2.970	0.000	3.086
6.001	Dr30	0.119	0.127	0.000	0.247
1.004	S10	2.967	0.909	0.000	3.876
7.000	Dr31	0.116	8.629	0.000	8.745
7.001	Dr32	0.123	0.094	0.000	0.217
8.000	CP20	0.227	3.175	0.000	3.402
8.001	CP19	0.195	0.469	0.000	0.665
1.005	S9.1	2.956	0.382	0.000	3.338
1.006	S9	2.938	0.621	0.000	3.560
9.000	Dr33	0.116	11.138	0.000	11.254
9.001	Dr34	0.123	0.102	0.000	0.226
10.000	CP23	0.170	2.877	0.000	3.047
10.001	CP22	0.202	1.091	0.000	1.293
1.007	S8.1	2.780	0.403	0.000	3.183
11.000	Dr35	0.116	12.683	0.000	12.799
11.001	Dr36	0.123	0.106	0.000	0.229
1.008	S8	2.818	0.361	0.000	3.179
12.000	CP26	0.170	3.233	0.000	3.403
12.001	CP25	0.206	0.397	0.000	0.603
12.002	AJ24	0.223	0.334	0.000	0.556
1.009	S7	2.784	0.240	0.000	3.025
13.000	Dr37	0.116	6.588	0.000	6.704
13.001	Dr38	0.124	0.286	0.000	0.411
1.010	S6	2.887	0.913	0.000	3.800
14.000	CP30	0.170	1.246	0.000	1.415
14.001	CP29	0.186	0.300	0.000	0.486
14.002	CP29.1	0.196	0.247	0.000	0.443
1.011	S5.1	2.709	0.288	0.000	2.996
1.012	S5	2.652	0.286	0.000	2.938
15.000	Dr01	0.116	5.556	0.000	5.672
15.001	Dr02	0.123	0.095	0.000	0.219
16.000	Dr03	0.116	10.638	0.000	10.754
16.001	Dr04	0.125	0.165	0.000	0.290
15.002	S46	2.006	1.033	0.000	3.039
17.000	Dr05	0.116	7.696	0.000	7.812
17.001	Dr06	0.168	0.146	0.000	0.314
18.000	CP50	0.190	2.342	0.000	2.532
18.001	CP49	0.170	1.637	0.000	1.807
18.002	CP48	0.202	0.296	0.000	0.498
18.003	AJ47	0.263	0.271	0.000	0.534
15.003	S45	1.764	0.811	0.000	2.575
19.000	CP55	0.170	2.227	0.000	2.397
19.001	CP54	0.214	2.031	0.000	2.245
19.002	CP53	0.313	0.308	0.000	0.621
19.003	CP52	0.395	0.114	0.000	0.509
19.004	J54	1.655	10.147	0.000	11.801
19.005	J55	1.713	0.217	0.000	1.930
19.006	S51	1.556	0.214	0.000	1.770

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Volume Summary (Static)

Pipe Number	USMH Name	Manhole Volume (m³)	Pipe Volume (m³)	Storage	Total Volume (m³)
				Structure Volume (m³)	
20.000	Dr07	0.116	12.595	0.000	12.711
20.001	Dr08	0.125	0.225	0.000	0.350
15.004	S44	1.583	2.066	0.000	3.649
15.005	S43	1.692	0.487	0.000	2.179
15.006	S42	1.812	0.396	0.000	2.207
21.000	CP59	0.170	2.041	0.000	2.211
21.001	CP58	0.207	1.259	0.000	1.466
21.002	CP57	0.232	2.561	0.000	2.793
21.003	CP56	0.377	0.134	0.000	0.511
15.007	S41	1.823	0.163	0.000	1.986
15.008	S40	1.802	0.228	0.000	2.030
22.000	X12	0.130	35.051	0.000	35.181
22.001	X13	0.080	0.266	0.000	0.347
22.002	S62	0.422	0.421	0.000	0.843
23.000	Dr09	0.116	11.664	0.000	11.780
23.001	Dr10	0.124	0.168	0.000	0.292
23.002	S64	0.501	0.384	0.000	0.885
24.000	Dr11	0.116	11.462	0.000	11.578
24.001	Dr12	0.126	0.153	0.000	0.279
24.002	S65	0.475	0.336	0.000	0.812
25.000	XX1	0.254	0.098	0.000	0.353
23.003	S63	1.597	0.737	0.000	2.334
26.000	AJDP01	0.464	0.904	0.000	1.368
26.001	AJDP02	0.593	0.892	0.000	1.485
26.002	AJDP03	0.719	0.612	0.000	1.331
27.000	AJDP05	0.464	0.716	0.000	1.180
27.001	AJDP06	0.565	0.585	0.000	1.151
26.003	AJDP04	0.806	0.516	0.000	1.322
22.003	S61	1.951	0.139	0.000	2.090
22.004	S61.1	1.919	135.606	0.000	137.525
22.005	S60	2.083	0.143	0.000	2.227
15.009	S39	2.105	0.394	0.000	2.499
15.010	S38	2.171	0.170	0.000	2.342
15.011	S37	2.228	0.516	0.000	2.744
15.012	S36	2.187	0.240	0.000	2.428
28.000	CP68	0.170	2.821	0.000	2.991
28.001	CP67	0.238	1.806	0.000	2.044
28.002	CP66	0.407	0.227	0.000	0.634
15.013	S35	2.162	0.251	0.000	2.413
29.000	Dr13	0.116	6.750	0.000	6.866
29.001	Dr14	0.123	0.196	0.000	0.319
30.000	CP71	0.170	2.250	0.000	2.420
30.001	CP70	0.297	2.514	0.000	2.811
30.002	CP69	0.387	0.231	0.000	0.618
31.000	CP75	0.177	2.693	0.000	2.870
32.000	CP76	0.170	1.961	0.000	2.130
31.001	CP74	0.206	0.299	0.000	0.505
31.002	AJ73	0.267	0.240	0.000	0.507
15.014	S34	2.248	0.874	0.000	3.122
33.000	Dr15	0.116	9.878	0.000	9.994
33.001	Dr16	0.124	0.186	0.000	0.309
15.015	S33	2.510	0.257	0.000	2.767
34.000	Dr17	0.116	8.752	0.000	8.868
34.001	Dr18	0.123	0.104	0.000	0.228
34.002	S77	1.233	1.779	0.000	3.012
35.000	Dr19	0.116	12.168	0.000	12.284
35.001	Dr20	0.194	0.115	0.000	0.309
36.000	CP81	0.170	2.371	0.000	2.541
36.001	CP80	0.210	0.303	0.000	0.513
36.002	AP79	0.271	0.239	0.000	0.510
15.016	S32	2.519	0.250	0.000	2.769
37.000	Dr21	0.464	9.350	0.000	9.814

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Volume Summary (Static)

Pipe Number	USMH Name	Manhole Volume (m³)	Pipe Volume (m³)	Storage Structure Volume (m³)	Total Volume (m³)
37.001	Dr22	0.456	0.206	0.000	0.702
15.017	S31	2.556	0.279	0.000	2.635
1.013	S4	2.773	0.120	0.000	2.693
1.014	S3	2.801	573.101	0.000	575.903
1.015	S2	2.226	0.175	0.000	2.401
Total		107.496	1005.095	0.000	1112.591

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Volume Summary (Static)

Length Calculations based on True Length

Pipe Number	US&B Name	Manhole Volume (m³)	Pipe Volume (m³)	Storage	Total Volume (m³)
				Structure Volume (m³)	
1.000	CP15	0.170	1.307	0.000	1.477
1.001	CP14	0.202	0.289	0.000	0.491
1.002	AJ13	0.169	0.236	0.000	0.408
2.000	Dr23	0.116	7.599	0.000	7.715
2.001	Dr24	0.123	0.098	0.000	0.221
3.000	Dr25	0.116	9.120	0.000	9.236
3.001	Dr26	0.123	0.174	0.000	0.297
1.003	S11	1.212	0.747	0.000	1.959
4.000	S17	3.613	0.801	0.000	4.414
4.001	S16	3.116	0.266	0.000	3.382
5.000	Dr27	0.116	7.401	0.000	7.517
5.001	Dr28	0.126	0.166	0.000	0.291
6.000	Dr29	0.116	2.257	0.000	2.373
6.001	Dr30	0.119	0.111	0.000	0.231
1.004	S10	2.967	0.862	0.000	3.828
7.000	Dr31	0.116	7.614	0.000	7.730
7.001	Dr32	0.123	0.078	0.000	0.201
8.000	CP20	0.227	3.091	0.000	3.318
8.001	CP19	0.195	0.454	0.000	0.649
1.005	S9.1	2.956	0.334	0.000	3.291
1.006	S9	2.938	0.574	0.000	3.512
9.000	Dr33	0.116	9.853	0.000	9.969
9.001	Dr34	0.123	0.087	0.000	0.210
10.000	CP23	0.170	2.802	0.000	2.971
10.001	CP22	0.202	1.055	0.000	1.257
1.007	S8.1	2.780	0.355	0.000	3.135
11.000	Dr35	0.116	11.219	0.000	11.335
11.001	Dr36	0.123	0.090	0.000	0.213
1.008	S8	2.818	0.313	0.000	3.131
12.000	CP26	0.170	3.158	0.000	3.327
12.001	CP25	0.206	0.387	0.000	0.593
12.002	AJ24	0.223	0.318	0.000	0.542
1.009	S7	2.784	0.193	0.000	2.977
13.000	Dr37	0.116	5.929	0.000	6.045
13.001	Dr38	0.124	0.270	0.000	0.395
1.010	S6	2.887	0.865	0.000	3.753
14.000	CP30	0.170	1.181	0.000	1.350
14.001	CP29	0.186	0.289	0.000	0.475
14.002	CP29.1	0.196	0.231	0.000	0.427
1.011	S5.1	2.709	0.240	0.000	2.949
1.012	S5	2.652	0.239	0.000	2.891
15.000	Dr01	0.116	4.919	0.000	5.035
15.001	Dr02	0.123	0.080	0.000	0.203
16.000	Dr03	0.116	9.612	0.000	9.728
16.001	Dr04	0.125	0.149	0.000	0.274
15.002	S46	2.006	0.985	0.000	2.992
17.000	Dr05	0.116	7.032	0.000	7.148
17.001	Dr06	0.168	0.130	0.000	0.298
18.000	CP50	0.190	2.272	0.000	2.463
18.001	CP49	0.170	1.562	0.000	1.731
18.002	CP48	0.202	0.285	0.000	0.487
18.003	AJ47	0.263	0.255	0.000	0.518
15.003	S45	1.764	0.763	0.000	2.527
19.000	CP55	0.170	2.152	0.000	2.321
19.001	CP54	0.214	1.936	0.000	2.149
19.002	CP53	0.313	0.297	0.000	0.610
19.003	CP52	0.395	0.098	0.000	0.493
19.004	J54	1.655	9.229	0.000	10.884
19.005	J55	1.713	0.169	0.000	1.882
19.006	S51	1.556	0.166	0.000	1.722

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Volume Summary (Static)

Pipe Number	USME Name	Manhole Volume (m³)	Pipe Volume (m³)	Storage Structure Volume (m³)	Total Volume (m³)
20.000	Dr07	0.116	11.444	0.000	11.560
20.001	Dr08	0.125	0.209	0.000	0.334
15.004	S44	1.583	2.016	0.000	3.602
15.005	S43	1.692	0.439	0.000	2.131
15.006	S42	1.812	0.348	0.000	2.160
21.000	CP59	0.170	1.966	0.000	2.135
21.001	CP58	0.207	1.167	0.000	1.374
21.002	CP57	0.232	2.460	0.000	2.692
21.003	CP56	0.377	0.119	0.000	0.495
15.007	S41	1.823	0.115	0.000	1.938
15.008	S40	1.802	0.181	0.000	1.982
22.000	X12	0.130	33.814	0.000	33.944
22.001	X13	0.080	0.243	0.000	0.323
22.002	S62	0.422	0.385	0.000	0.807
23.000	Dr09	0.116	10.368	0.000	10.484
23.001	Dx10	0.124	0.157	0.000	0.281
23.002	S64	0.501	0.348	0.000	0.849
24.000	Dr11	0.116	10.461	0.000	10.577
24.001	Dr12	0.126	0.142	0.000	0.268
24.002	S65	0.475	0.301	0.000	0.776
25.000	XX1	0.254	0.091	0.000	0.346
23.003	S63	1.597	0.690	0.000	2.287
26.000	AJDP01	0.464	0.857	0.000	1.320
26.001	AJDP02	0.593	0.845	0.000	1.437
26.002	AJDP03	0.719	0.564	0.000	1.283
27.000	AJDP05	0.464	0.669	0.000	1.132
27.001	AJDP06	0.565	0.538	0.000	1.103
26.003	AJDP04	0.806	0.468	0.000	1.275
22.003	S61	1.951	0.091	0.000	2.042
22.004	S61.1	1.919	129.824	0.000	131.743
22.005	S60	2.083	0.096	0.000	2.179
15.009	S39	2.105	0.346	0.000	2.451
15.010	S38	2.171	0.123	0.000	2.294
15.011	S37	2.228	0.468	0.000	2.696
15.012	S36	2.187	0.193	0.000	2.380
28.000	CP68	0.170	2.746	0.000	2.915
28.001	CP67	0.238	1.724	0.000	1.962
28.002	CP66	0.407	0.212	0.000	0.618
15.013	S35	2.162	0.203	0.000	2.365
29.000	Dr13	0.116	5.940	0.000	6.056
29.001	Dr14	0.123	0.180	0.000	0.303
30.000	CP71	0.170	2.174	0.000	2.344
30.001	CP70	0.297	2.427	0.000	2.724
30.002	CP69	0.387	0.215	0.000	0.602
31.000	CP75	0.177	2.615	0.000	2.792
32.000	CP76	0.170	1.885	0.000	2.055
31.001	CP74	0.206	0.289	0.000	0.495
31.002	AJ73	0.267	0.225	0.000	0.491
15.014	S34	2.248	0.826	0.000	3.074
33.000	Dr15	0.116	8.820	0.000	8.936
33.001	Dr16	0.124	0.170	0.000	0.293
15.015	S33	2.510	0.209	0.000	2.719
34.000	Dr17	0.116	7.742	0.000	7.858
34.001	Dr18	0.123	0.088	0.000	0.212
34.002	S77	1.233	1.731	0.000	2.964
35.000	Dr19	0.116	10.764	0.000	10.880
35.001	Dr20	0.194	0.099	0.000	0.293
36.000	CP81	0.170	2.296	0.000	2.465
36.001	CP80	0.210	0.292	0.000	0.502
36.002	AP79	0.271	0.223	0.000	0.494
15.016	S32	2.519	0.202	0.000	2.721
37.000	Dr21	0.464	7.396	0.000	7.859

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Volume Summary (Static)

Pipe Number	URGH Name	Manhole Volume (m³)	Pipe Volume (m³)	Storage	Total Volume (m³)
				Structure Volume (m³)	
37.001	Dr22	0.496	0.184	0.000	0.681
15.017	S31	2.556	0.231	0.000	2.787
1.013	S4	2.773	0.072	0.000	2.845
1.014	S3	2.801	545.395	0.000	548.197
1.015	S2	2.226	0.128	0.000	2.353
Total		107.496	944.996	0.000	1052.493

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Summary of Critical Results by Maximum Level (Rank 1) for Storm

Simulation Criteria

Areal Reduction Factor 1.000 Hot Start Level (mm) 0 Foul Sewage per hectare (l/s) 0.000 MADD Factor * 10m³/ha Storage 2.000 Flow per Person per Day (l/per/day) 0.000
Hot Start (mins) 0 Manhole Headless Coeff (Global) 0.500 Additional Flow - % of Total Flow 0.000 Inlet Coefficient 0.800
Number of Input Hydrographs 0 Number of Online Controls 22 Number of Offline Controls 0 Number of Storage Structures 42 Number of Time/Area Diagrams 61 Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model FSR Region Scotland and Ireland M5-60 (mm) 17.700 Ratio R 0.270 Cv (Summer) 0.750 Cv (Winter) 0.840

Margin for Flood Risk Warning (mm) 300.0 DTS Status ON Inertia Status ON
Analysis Timestep 2.5 Second Increment (Extended) DVD Status ON

Profile(s) Summer and Winter
Duration(s) (mins) 15, 30, 60, 120, 240, 360, 480, 960, 1440
Return Period(s) (years) 1, 30, 100
Climate Change (%) 10, 10, 10

FN	US/ME Name	Storm	Return Period	Climate Change	First (X) Surge	First (Y) Flood	First (Z) Overflow	Overflow Act.	Water Level (m)	Surcharged Depth (m)	Flooded Volume (m ³)	Flow / Cap. (l/s)	Half Drain Time (mins)	Pipe Flow (l/s)	Status	Level Exceeded
1.000	CP15	30	Winter	100	+10%				94.121	-0.479	0.000	0.14	14	1.8	OK	
1.001	CP14	30	Winter	100	+10%				93.925	-0.111	0.000	0.15		1.8	OK	
1.002	AJ13	30	Winter	100	+10%				93.830	-0.121	0.000	0.08		1.8	OK	
2.000	Dr23	120	Winter	100	+10%				94.336	-0.154	0.000	0.01	67	1.3	FLOOD RISK	
2.001	Dr24	120	Winter	100	+10%	100/60	Winter		94.336	0.021	0.000	0.04		0.7	FLOOD RISK	
3.000	Dr25	120	Winter	100	+10%				94.344	-0.146	0.000	0.01	78	1.5	FLOOD RISK	
3.001	Dr26	120	Winter	100	+10%	100/60	Winter		94.344	0.028	0.000	0.04		0.8	FLOOD RISK	
1.003	S11	15	Winter	100	+10%				93.563	-0.148	0.000	0.26		8.6	OK	
4.000	S17	120	Winter	100	+10%	30/15	Summer		93.369	0.838	0.000	0.15		5.0	SURCHARGED	
4.001	S16	120	Winter	100	+10%	30/15	Summer		93.359	0.935	0.000	0.16		4.8	SURCHARGED	
5.000	Dr27	15	Summer	1	+10%				94.640	-0.300	0.000	0.00		0.0	OK	
5.001	Dr28	15	Summer	1	+10%				94.606	-0.150	0.000	0.00		0.0	OK	
6.000	Dr29	60	Winter	100	+10%				94.813	-0.152	0.000	0.01	27	1.3	FLOOD RISK	
6.001	Dr30	60	Winter	100	+10%	100/60	Winter		94.813	0.010	0.000	0.20		0.7	FLOOD RISK	
1.004	S10	120	Winter	100	+10%	30/15	Summer		93.354	0.969	0.000	0.30		10.2	SURCHARGED	
7.000	Dr31	120	Winter	100	+10%				94.594	-0.146	0.000	0.01	68	1.3	FLOOD RISK	
7.001	Dr32	120	Winter	100	+10%	100/60	Winter		94.594	0.030	0.000	0.23		0.8	FLOOD RISK	
8.000	CP20	30	Winter	100	+10%				94.360	-0.583	0.000	0.07	11	1.4	OK	
8.001	CP19	60	Winter	100	+10%				94.256	-0.054	0.000	0.73		1.4	OK	
1.005	S9.1	120	Winter	100	+10%	30/15	Summer		93.334	1.063	0.000	2.10		11.5	SURCHARGED	
1.006	S9	120	Winter	100	+10%	1/15	Winter		93.131	0.908	0.000	2.39		13.1	SURCHARGED	
9.000	Dr33	120	Winter	100	+10%				94.341	-0.149	0.000	0.01	81	1.6	FLOOD RISK	
9.001	Dr34	120	Winter	100	+10%	100/60	Winter		94.342	0.028	0.000	0.22		0.8	FLOOD RISK	
10.000	CP23	60	Winter	100	+10%				94.107	-0.493	0.000	0.11	20	1.9	OK	
10.001	CP22	60	Winter	100	+10%				93.977	-0.134	0.000	0.34		1.9	OK	
11.000	Dr35	240	Winter	100	+10%				94.200	0.843	0.000	1.21		7.0	SURCHARGED	
11.001	Dr36	240	Winter	100	+10%	100/60	Winter		94.200	-0.140	0.000	0.00	113	1.3	FLOOD RISK	
1.008	S8	960	Winter	100	+10%	30/60	Summer		92.978	0.036	0.000	0.23		0.8	FLOOD RISK	
12.000	CP26	60	Winter	100	+10%				92.978	0.884	0.000	0.25		7.5	SURCHARGED	
12.001	CP26	60	Winter	100	+10%				93.964	-0.486	0.000	0.13	22	2.1	OK	
12.002	CP25	60	Winter	100	+10%				93.764	-0.108	0.000	0.18		2.1	OK	
12.003	AJ24	60	Winter	100	+10%				93.652	-0.108	0.000	0.18		2.1	OK	
1.009	S7	960	Winter	100	+10%	30/30	Winter		92.976	0.927	0.000	0.28		7.9	SURCHARGED	
13.000	Dr37	120	Winter	100	+10%				94.101	-0.089	0.000	0.01	68	1.3	FLOOD RISK	
13.001	Dr38	120	Winter	100	+10%	30/60	Summer		94.101	0.091	0.000	0.04		0.9	FLOOD RISK	
1.010	S6	960	Winter	100	+10%	1/480	Winter		92.975	0.956	0.000	0.28		9.3	SURCHARGED	
14.000	CP30	60	Winter	100	+10%				93.685	-0.465	0.000	0.16	22	2.1	OK	
14.001	CP29	60	Winter	100	+10%				93.535	-0.107	0.000	0.18		2.1	OK	
14.002	CP29.1	60	Winter	100	+10%				93.450	-0.107	0.000	0.18		2.1	OK	
1.011	S5.1	960	Winter	100	+10%	1/120	Winter		92.971	1.067	0.000	0.34		9.7	SURCHARGED	
1.012	S5	960	Winter	100	+10%	1/60	Winter		92.969	1.101	0.000	0.33		9.6	SURCHARGED	
15.000	Dr01	120	Winter	100	+10%				94.180	-0.085	0.000	0.01	60	1.4	FLOOD RISK	

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Summary of Critical Results by Maximum Level (Rank 1) for Storm

PN	US/HR Name	Storm	Return Period	Climate Change	First (X) Surchage	First (Y) Flood	First (Z) Overflow	Overflow Act.	Water Level (m)	Surcharged Depth (m)	Flooded Volume (m³)	Flow / Overflow Cap. (l/s)	Half Drain Time (mins)	Pipe Flow (l/s)	Status	Level Exceeded
15.001	Dr02	120	Winter	100	+10%	30/60	Summer		94.180	0.091	0.000	0.05		0.9	FLOOD RISK	
16.000	Dr03	120	Winter	100	+10%				94.144	-0.121	0.000	0.01		86	1.4	FLOOD RISK
16.001	Dr04	120	Winter	100	+10%	30/120	Winter		94.144	0.060	0.000	0.04			0.8	FLOOD RISK
15.002	S46	120	Winter	100	+10%	100/60	Winter		93.052	0.164	0.000	0.15			4.9	SURCHARGED
17.000	Dr05	60	Winter	100	+10%				93.838	-0.202	0.000	0.01		30	1.5	OK
17.001	Dr06	60	Winter	100	+10%	1/60	Winter		93.838	0.133	0.000	0.05			1.0	SURCHARGED
18.000	CP50	60	Winter	100	+10%				93.810	-0.441	0.000	0.13		20	1.9	OK
18.001	CP49	60	Winter	100	+10%				93.697	-0.453	0.000	0.18		19	3.1	OK
18.002	CP48	60	Winter	100	+10%				93.487	-0.098	0.000	0.26			3.1	OK
18.003	AJ47	60	Winter	100	+10%				93.403	-0.098	0.000	0.26			3.1	OK
15.003	S45	120	Winter	100	+10%	100/30	Winter		93.047	0.289	0.000	0.33			10.9	SURCHARGED
19.000	CP55	30	Winter	100	+10%				94.075	-0.525	0.000	0.07		14	1.8	OK
19.001	CP54	60	Winter	100	+10%				93.749	-0.626	0.000	0.11		18	3.6	OK
19.002	CP53	60	Winter	100	+10%	100/30	Winter		93.491	-0.074	0.000	1.05			3.6	SURCHARGED
19.003	CP52	60	Winter	100	+10%	30/60	Winter		93.173	-0.046	0.000	1.35			3.6	SURCHARGED
19.004	J54	120	Winter	100	+10%				93.056	-1.319	0.000	0.01		16	2.9	OK
19.005	J55	120	Winter	100	+10%	100/30	Winter		93.055	0.345	0.000	0.72			3.9	SURCHARGED
19.006	S51	120	Winter	100	+10%	30/120	Winter		93.044	0.361	0.000	0.78			4.3	SURCHARGED
20.000	Dr07	240	Winter	100	+10%				93.752	-0.033	0.000	0.01		141	1.5	FLOOD RISK
20.001	Dr08	240	Winter	100	+10%	30/30	Winter		93.753	0.151	0.000	0.05			1.0	FLOOD RISK
15.004	S44	120	Winter	100	+10%	30/120	Winter		93.033	0.377	0.000	0.41			14.6	SURCHARGED
15.005	S43	960	Winter	100	+10%	30/15	Summer		93.012	0.616	0.000	0.20			6.2	SURCHARGED
15.006	S42	960	Winter	100	+10%	30/15	Summer		93.010	0.675	0.000	0.22			6.8	SURCHARGED
21.000	CP59	60	Winter	100	+10%				93.472	-0.423	0.000	0.24		27	4.0	OK
21.001	CP58	60	Winter	100	+10%				93.340	-0.555	0.000	0.18		24	3.9	OK
21.002	CP57	60	Winter	100	+10%				93.248	-0.625	0.000	0.16		25	3.9	OK
21.003	CP56	960	Winter	100	+10%	100/60	Summer		93.008	0.296	0.000	0.09			1.0	SURCHARGED
15.007	S41	960	Winter	100	+10%	30/15	Summer		93.008	0.723	0.000	0.32			8.1	SURCHARGED
15.008	S40	960	Winter	100	+10%	30/15	Summer		93.006	0.741	0.000	0.41			11.4	SURCHARGED
22.000	X12	360	Winter	100	+10%				93.483	-0.181	0.000	0.01		219	1.0	OK
22.001	X13	360	Winter	100	+10%				93.484	-0.087	0.000	0.02			0.7	FLOOD RISK
22.002	S62	960	Winter	100	+10%	30/60	Summer		93.138	0.765	0.000	0.02			0.6	SURCHARGED
23.000	Dr09	120	Winter	100	+10%				93.919	-0.121	0.000	0.01		92	1.4	FLOOD RISK
23.001	Dr10	120	Winter	100	+10%	30/120	Winter		93.919	0.056	0.000	0.04			0.8	FLOOD RISK
23.002	S64	960	Winter	100	+10%	30/120	Summer		93.138	0.677	0.000	0.02			0.6	SURCHARGED
24.000	Dr11	120	Winter	100	+10%				93.681	-0.119	0.000	0.01		91	1.4	FLOOD RISK
24.001	Dr12	120	Winter	100	+10%	30/60	Winter		93.681	0.065	0.000	0.04			0.8	FLOOD RISK
24.002	S65	960	Winter	100	+10%	30/120	Summer		93.138	0.683	0.000	0.02			0.7	SURCHARGED
25.000	XX1	960	Winter	100	+10%	100/240	Winter		93.137	0.337	0.000	0.00			0.0	SURCHARGED
23.003	S63	960	Winter	100	+10%	30/60	Winter		93.137	0.724	0.000	0.04			1.2	SURCHARGED
26.000	AJDP01	60	Winter	100	+10%				93.392	-0.173	0.000	0.12			4.1	OK
26.001	AJDP02	60	Winter	100	+10%				93.278	-0.173	0.000	0.12			4.1	OK
26.002	AJDP03	60	Winter	100	+10%				93.167	-0.172	0.000	0.13			4.1	OK
27.000	AJDP05	60	Winter	100	+10%				93.393	-0.172	0.000	0.13			4.1	OK
27.001	AJDP06	60	Winter	100	+10%				93.290	-0.185	0.000	0.07			4.1	OK
26.003	AJDP04	960	Winter	100	+10%				93.137	-0.125	0.000	0.07			2.3	OK
22.003	S61	960	Winter	100	+10%	1/360	Winter		93.137	0.817	0.000	0.15			3.9	SURCHARGED
22.004	S61.1	960	Winter	100	+10%				93.136	-0.304	0.000	0.00		1092	5.2	OK
22.005	S60	960	Winter	100	+10%	1/120	Winter		93.136	0.881	0.000	0.10			0.9	SURCHARGED
15.009	S39	960	Winter	100	+10%	30/15	Summer		93.004	0.768	0.000	0.38			11.5	SURCHARGED
15.010	S38	960	Winter	100	+10%	30/15	Summer		93.000	0.814	0.000	0.45			11.5	SURCHARGED
15.011	S37	960	Winter	100	+10%	30/15	Summer		92.998	0.833	0.000	0.39			12.4	SURCHARGED
15.012	S36	960	Winter	100	+10%	30/15	Summer		92.993	0.893	0.000	0.44			12.3	SURCHARGED
28.000	CP68	60	Winter	100	+10%				93.666	-0.484	0.000	0.13		23	2.1	OK
28.001	CP67	60	Winter	100	+10%				93.488	-0.472	0.000	0.22		21	4.0	OK
28.002	CP66	960	Winter	100	+10%	30/15	Summer		92.994	0.372	0.000	0.50			0.9	SURCHARGED
15.013	S35	960	Winter	100	+10%	30/15	Summer		92.990	0.920	0.000	0.46			13.1	SURCHARGED
29.000	Dr13	120	Winter	100	+10%				93.778	-0.112	0.000	0.01		64	1.4	FLOOD RISK
29.001	Dr14	120	Winter	100	+10%	30/60	Winter		93.778	0.063	0.000	0.04			0.8	FLOOD RISK
30.000	CP71	60	Winter	100	+10%				93.842	-0.493	0.000	0.11		20	1.9	OK
30.001	CP70	60	Winter	100	+10%				93.465	-0.505	0.000	0.21		20	4.0	OK

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Summary of Critical Results by Maximum Level (Rank 1) for Storm

US/ME	Return Climate	First (X)	First (X)	First (Z)	Overflow	Water Level	Surcharged Depth	Flooded Volume	Flow / Overflow	Half Drain Time	Pipe Flow	Level Exceeded
FN Name Storm	Period Change	Surcharge	Flood	Overflow	Act.	(m)	(m)	(m³)	Cap. (l/s)	(mins)	(l/s)	Status
30.002 CP69 960 Winter	100 +10%	30/15 Summer				92.991	0.324	0.000	0.52		1.0	SURCHARGED
31.000 CP75 15 Summer	1 +10%					93.424	-0.626	0.000	0.00		0.0	OK
32.000 CP76 30 Winter	100 +10%					93.496	-0.504	0.000	0.10	11	1.6	OK
31.001 CP74 30 Winter	100 +10%					93.377	-0.045	0.000	0.83		1.6	OK
31.002 A773 30 Winter	100 +10%					93.292	-0.045	0.000	0.83		1.5	OK
15.014 S34 960 Winter	100 +10%	30/15 Summer				92.987	0.949	0.000	0.47		15.7	SURCHARGED
33.000 Dr15 120 Winter	100 +10%					93.824	-0.116	0.000	0.01	83	1.3	FLOOD RISK
33.001 Dr16 120 Winter	100 +10%	30/120 Winter				93.824	0.062	0.000	0.04		0.8	FLOOD RISK
15.015 S33 960 Winter	100 +10%	1/120 Winter				92.979	1.051	0.000	0.58		16.2	SURCHARGED
34.000 Dr17 120 Winter	100 +10%					94.109	-0.116	0.000	0.01	77	1.6	FLOOD RISK
34.001 Dr18 120 Winter	100 +10%	30/120 Winter				94.109	0.060	0.000	0.04		0.8	FLOOD RISK
34.002 S77 15 Winter	100 +10%					93.220	-0.140	0.000	0.30		10.6	OK
35.000 Dr19 120 Winter	100 +10%					93.609	-0.166	0.000	0.01	55	2.2	FLOOD RISK
35.001 Dr20 120 Winter	100 +10%	1/15 Summer				93.609	0.259	0.000	0.35		1.2	FLOOD RISK
36.000 CP81 60 Winter	100 +10%					93.661	-0.489	0.000	0.12	21	2.0	OK
36.001 CP80 60 Winter	100 -10%	100/60 Winter				93.558	0.002	0.000	1.06		2.0	SURCHARGED
36.002 AP79 60 Winter	100 -10%					93.452	-0.018	0.000	1.00		1.9	OK
15.016 S32 960 Winter	100 +10%	1/120 Summer				92.975	1.079	0.000	0.69		19.4	SURCHARGED
37.000 Dr21 120 Winter	100 +10%					93.941	-0.099	0.000	0.01	80	1.3	FLOOD RISK
37.001 Dr22 120 Winter	100 +10%	30/60 Winter				93.941	0.080	0.000	0.08		0.9	FLOOD RISK
15.017 S31 960 Winter	100 +10%	1/60 Winter				92.971	1.106	0.000	0.69		19.9	SURCHARGED
1.013 S4 960 Winter	100 +10%	1/60 Winter				92.967	1.135	0.000	1.09		29.4	SURCHARGED
1.014 S3 960 Winter	100 +10%					92.964	-0.384	0.000	0.00	999	29.3	OK
1.015 S2 960 Winter	100 +10%	1/15 Summer				92.963	1.291	0.000	0.21		5.4	SURCHARGED