



Thor Design & Management

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Proposed 6No. Additional Dwellings @ St. Finian's Way, Newcastle, Co. Dublin.

Sewer Design

Date: 8th February 2022

*The proposed foul sewer design calculations have been designed in accordance with
Irish Water Standards*



Met Stream
Return Period Rainfall Depths for sliding Durations
Irish Grid: Easting: 29902, Northing: 22921.

DURATION	5month	1 year	2	3	4	5	10	25	30	50	75	100	150	200	250	500
5 mins	2.5	3.6	4.3	5.2	5.9	6.5	8.2	10.3	11.7	13.7	15.5	17.0	19.2	20.9	22.4	N/A
10 mins	3.4	5.0	5.9	7.3	8.1	9.0	11.5	14.4	16.3	19.1	21.7	23.6	26.7	29.2	31.2	N/A
15 mins	4.0	5.9	7.0	8.6	9.7	10.6	13.5	16.9	19.2	22.5	25.5	27.8	31.5	34.3	36.7	N/A
30 mins	5.3	7.7	9.0	11.1	12.5	13.6	17.2	21.5	24.3	28.3	32.0	34.8	39.3	42.8	45.7	N/A
1 hour	6.9	10.0	11.7	14.1	15.1	17.4	22.0	27.2	30.7	35.7	40.2	41.7	49.1	53.3	56.8	N/A
2 hours	9.2	13.1	15.2	18.4	20.7	22.4	28.0	34.5	38.9	45.0	50.4	51.7	61.3	66.4	70.7	N/A
3 hours	10.7	15.3	17.7	21.4	23.9	25.9	32.3	39.7	44.6	52.5	57.6	58.4	69.8	75.6	80.4	N/A
4 hours	12.0	17.0	19.7	23.8	26.6	28.7	35.9	43.8	49.1	56.6	63.3	64.5	76.6	82.6	88.0	N/A
6 hours	14.1	19.9	23.0	27.5	30.8	33.2	41.3	50.4	56.4	64.8	72.4	74.2	87.2	94.2	100.0	N/A
9 hours	16.5	23.2	26.7	32.1	35.7	38.5	47.6	57.9	64.7	74.2	82.7	85.2	99.3	107.1	113.6	N/A
12 hours	18.5	25.9	29.8	35.6	39.6	42.7	52.6	63.9	71.3	81.6	90.9	93.0	108.9	117.4	124.4	N/A
18 hours	21.7	30.2	34.7	41.4	45.9	49.4	60.7	73.4	81.8	93.4	103.8	106.8	124.1	133.5	141.4	N/A
24 hours	24.4	33.7	38.6	46.0	50.9	54.7	67.1	81.0	90.1	102.8	114.1	117.8	138.0	146.9	154.8	N/A
2 days	30.5	41.0	46.5	54.5	59.9	63.9	77.0	91.4	100.7	113.6	124.9	133.5	156.6	166.7	175.0	203.5
3 days	35.6	47.1	53.2	61.6	67.2	71.5	86.3	100.3	109.9	123.1	134.6	143.3	166.7	175.9	184.3	212.9
4 days	40.1	52.4	58.7	67.8	73.7	78.3	92.6	108.1	118.0	131.6	143.3	152.2	165.7	175.9	184.3	212.9
6 days	48.1	61.8	69.7	78.7	85.1	90.0	105.3	121.9	131.3	146.5	158.8	168.0	181.9	192.4	201.0	230.1
8 days	55.3	70.2	77.7	88.3	95.2	100.4	116.7	134.0	144.9	161.7	172.4	182.0	196.3	207.1	215.9	245.5
10 days	61.9	77.9	85.8	97.1	104.4	109.9	127.0	145.1	156.4	173.7	184.8	194.6	209.4	220.4	229.4	259.7
12 days	68.2	85.1	93.5	105.4	113.0	118.7	136.6	155.3	167.1	182.9	196.3	206.4	221.5	232.8	242.0	272.8
16 days	79.9	98.6	107.7	120.6	128.9	135.1	154.2	174.2	186.5	203.3	217.4	229.0	243.7	255.5	265.0	296.8
20 days	90.8	111.1	120.9	134.7	143.5	150.1	170.3	191.4	204.5	221.9	236.6	247.6	263.9	276.1	286.0	318.7
25 days	103.8	125.7	136.4	151.2	160.5	167.6	189.1	211.4	225.1	243.4	258.8	270.3	287.3	299.9	310.1	343.9

N/A Data not available
These values are derived from a Depth Duration Frequency (DDF) Model
For details refer to:
Fitzgerald D. L. (2007). Estimates of Point Rainfall Frequencies, Technical Note No. 61. Met Stream, Dublin.
Available for download at www.met.ie/climate/data/products/Estimation-of-Point-Rainfall-Frequencies_TN61.pdf

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Date 7th Feb 2021

Development: Additional 6No. Dwellings @ St. Fiaian's Way, Newcastle, Co. Dublin

Developer: Pavement Homes Ltd

PROPOSED HOUSING DEVELOPMENT

Surface Water Design

Pipe Coefficient k=0.06 (new PVC Pipe), Vmin=0.8m/s, Vmax=4.0m/s, V(ideal)=1.8m/s

Area for Road Section	from	L - M - N - P								
Manholes:	from	S0	to	S1						
Distance between Manholes	=	50.215 m								
Road	=	434.68 m	x	1.0 m	=	434.68 m ²	x	95.00% (impermeability)	=	412.946 m ²
Footpath	=	53.52 m	x	2.0 m	=	107.04 m ²	x	95.00% (impermeability)	=	101.688 m ²
Front paved areas	=	345.77 m ²	x	1.0 No	=	345.77 m ²	x	100.00% (impermeability)	=	345.77 m ²
Driveway	=	0.00 m ²	x	0.0 No	=	0.00 m ²	x	100.00% (impermeability)	=	0 m ²
Area of Roofs 1	=	57.87 m ²	x	6.0 No	=	347.22 m ²	x	100.00% (impermeability)	=	347.22 m ²
Area of Roofs 2	=	0.00 m ²	x	8.0 No	=	0.00 m ²	x	100.00% (impermeability)	=	0 m ²
Green Area	=	213.54 m ²	x	1.0 No	=	213.54 m ²	x	10.00% (impermeability)	=	21.354 m ²
									Total	1228.98 m²

Taking Rain Intensity of 50 mm per Hour

$$\text{Vol of Water try} = \frac{1228.98 \text{ m}^2 \times 0.05 \text{ m}}{225 \text{ diameter pipe at fall}} = 61.45 \text{ m}^3/\text{hr} = 17.07 \text{ l/s}$$

$$\text{Therefore } v = \frac{17.07 \text{ l/s}}{1.15 \text{ m/s}} = 1.15 \text{ m/s}$$

$$\text{Total Area} = 1228.98 \text{ m}^2 = 0.30 \text{ Acres}$$

$$Q = 60.5 \times A_p \times R$$

Where A_p = Impermeable Area (Acres)

R = Rate of Discharge (inches).

$$\text{Time Taken for Surface Water to travel between Manholes} = \frac{50.215}{1.15} = 43.67 = 0.73 \text{ mins}$$

$$\text{Time of Concentration (T)} = 0.73 + 3.0 = 3.73 \text{ min}$$

$$R = \frac{30}{10 + T} = \frac{30}{10 + 3.72775362} = 2.19$$

$$Q(\text{actual}) = 60.5 \times 0.30368 \text{ acres} \times 2.19 \text{ inches} = 40.15 \text{ ft}^3/\text{min} = 18.95 \text{ l/s}$$

$$\text{Therefore use } 225 \text{ diameter @ fall } 1:120 = 0.833\% \text{ OK}$$

Pipe Coefficient k=0.06 (new PVC Pipe), Vmin=0.8m/s, Vmax=4.0m/s, V(ideal)=1.8m/s

Area for Road Section	from	J - K - L - M								
Manholes:	from	S1	to	S2						
Distance between Manholes	=	43.865 m								
Road	=	336.48 m	x	1.0 m	=	336.48 m ²	x	95.00% (impermeability)	=	319.656 m ²
Footpath	=	55.56 m	x	2.0 m	=	111.12 m ²	x	95.00% (impermeability)	=	105.564 m ²
Front paved areas	=	553.00 m ²	x	1.0 No	=	553.00 m ²	x	100.00% (impermeability)	=	553 m ²
Driveway	=	0.00 m ²	x	0.0 No	=	0.00 m ²	x	100.00% (impermeability)	=	0 m ²
Area of Roofs 1	=	83.99 m ²	x	1.0 No	=	83.99 m ²	x	100.00% (impermeability)	=	83.99 m ²
Area of Roofs 2	=	57.90 m ²	x	8.0 No	=	463.20 m ²	x	100.00% (impermeability)	=	463.2 m ²
Green Area	=	456.30 m ²	x	1.0 No	=	456.30 m ²	x	10.00% (impermeability)	=	45.63 m ²
									Total	1571.04 m²

Taking Rain Intensity of 50 mm per Hour



$$\text{Vol of Water try} = \frac{1571.04 \text{ m}^2}{225 \text{ diameter pipe at fall}} \times \frac{0.05 \text{ m}}{120} = \frac{78.55 \text{ m}^3/\text{hr}}{\text{Therefore } v=} = \frac{21.82 \text{ l/s}}{1.33 \text{ m/s}}$$

$$\text{Total Area} = 1571.04 \text{ m}^2 = 0.39 \text{ Acres}$$

$$Q = 60.5 \times A_p \times R$$

Where A_p = Impermeable Area (Acres)

R = Rate of Discharge (inches).

$$\text{Time Taken for Surface Water to travel between Manholes} = \frac{43.865}{1.33} = 32.98 = 0.55 \text{ mins}$$

$$\text{Time of Concentration (T)} = 0.55 + 3.0 = 3.55 \text{ min}$$

$$R = \frac{30}{10 + T} = \frac{30}{10 + 3.54968672} = 2.21$$

$$Q(\text{actual}) = 60.5 \times 0.3882 \text{ acres} \times 2.21 \text{ inches} = 52.00 \text{ ft}^3/\text{min} = 24.54 \text{ l/s} + 18.95$$

$$\text{Therefore use } 225 \text{ diameter @ fall } 1 \text{ } 120 = 0.833\% \text{ OK} = 43.49 \text{ L/s}$$

100
10000

Surface Water Design

Pipe Coefficient k=0.06 (new PVC Pipe), Vmin=0.8m/s, Vmax=4.0m/s, V(ideal)=1.8m/s

Area for Road Section: from G - H - J
 Manholes: from S2 to S3

Distance between Manholes = 43.931 m

Road	=	199.64 m	x	1.0 m	=	199.64 m ²	x	95.00% (impermeability)	=	189.658 m ²
Footpath	=	36.52 m	x	2.0 m	=	73.04 m ²	x	95.00% (impermeability)	=	69.388 m ²
Front Paved Area	=	311.74 m ²	x	1.0 No	=	311.74 m ²	x	100.00% (impermeability)	=	311.74 m ²
Area of Roofs	=	57.90 m ²	x	4.0 No	=	231.60 m ²	x	100.00% (impermeability)	=	231.6 m ²
Grass Verge	=	14.40 m ²	x	1.0 No	=	14.40 m ²	x	10.00% (impermeability)	=	1.44 m ²
Area of Roofs	=	72.87 m ²	x	1.0 No	=	72.87 m ²	x	100.00% (impermeability)	=	72.87 m ²
Green Area	=	519.23 m ²	x	1.0 No	=	519.23 m ²	x	10.00% (impermeability)	=	51.923 m ²

Total 928.62 m²

Taking Rain Intensity of 50 mm per Hour

$$\text{Vol of Water try} = \frac{928.62 \text{ m}^2}{225 \text{ diameter pipe at fall}} \times 0.05 \text{ m} = 46.43 \text{ m}^3/\text{hr} = 12.90 \text{ l/s} + 43.49 \text{ l/s} = 56.39 \text{ l/s}$$

Therefore v= 1.3 m/s

$$\text{Total Area} = 928.62 \text{ m}^2 = 0.23 \text{ Acres}$$

$$Q = 60.5 \times A_p \times R$$

Where A_p = Impermeable Area (Acres)

R = Rate of Discharge (inches).

$$\text{Time Taken for Surface Water to travel between Manholes} = \frac{43.931}{1.3} = 33.79 = 0.56 \text{ mins}$$

$$\text{Time of Concentration (T)} = 0.56 + 3.0 = 3.56 \text{ min}$$

$$R = \frac{30}{10 + T} = \frac{30}{10 + 3.56321795} = 2.21$$

$$Q(\text{actual}) = 60.5 \times 0.22946 \text{ acres} \times 2.21 \text{ inches} = 30.71 \text{ ft}^3/\text{min} = 14.49 \text{ l/s} + 57.98 \text{ l/s}$$

Therefore use 225 diameter @ fall 1: 160 = 0.625% OK

TOTAL IMPERMEABLE AREA = 3786.64Sq.m



Surface Water Design

Pipe Coefficient k=0.06 (new PVC Pipe), Vmin=0.8m/s, Vmax=4.0m/s, V(ideal)=1.8m/s

Area for Road Section: from G to E
Manholes from S3 to S4

Distance between Manholes = 34.7 m

Road	=	190.24 m	x	1.0 m	=	190.24 m ²	x	95.00% (impermeability)	=	180.728 m ²
Footpath & Paving	=	48.90 m	x	1.0 m	=	48.90 m ²	x	95.00% (impermeability)	=	46.455 m ²
Grass Verge	=	49.10 m ²	x	1.0 No	=	49.10 m ²	x	10.00% (impermeability)	=	4.91 m ²
Car Park spaces	=	0.00 m ²	x	1.0 No	=	0.00 m ²	x	100.00% (impermeability)	=	0 m ²
Area of Roofs	=	0.00 m ²	x	1.0 No	=	0.00 m ²	x	100.00% (impermeability)	=	0 m ²
Area of Roofs	=	0.00 m ²	x	0.0 No	=	0.00 m ²	x	100.00% (impermeability)	=	0 m ²
Green Area	=	195.60 m ²	x	1.0 No	=	195.60 m ²	x	10.00% (impermeability)	=	19.56 m ²
Total										
<u><u>251.65 m²</u></u>										

Taking Rain Intensity of 50 mm per Hour

Vol of Water = 251.65 m² x 0.05 m = 12.58 m³/hr = 3.50 l/s + 7.0 l/s ATTENUATED
 try 225 diameter pipe at fall 1:160 Therefore v= 0.85 m/s = 10.5 l/s

Total Area = 251.65 m² = 0.06 Acres

Q = 60.5 x Ap x R

Where Ap = Impermeable Area (Acres)
R = Rate of Discharge (inches).

Time Taken for Surface Water to travel between Manholes = $\frac{34.7}{0.85} = 40.82 = 0.68 \text{ mins}$

Time of Concentration (T) = 0.68 + 3.0 = 3.68 min

R = $\frac{30}{10 + T} = \frac{30}{10 + 3.68039216} = 2.19$

Q(actual) = 60.5 x 0.06218 acres x 2.19 inches = 8.25 ft³/min = 3.89 l/s + 7.0 l/s = 10.89 l/s

Therefore use 225 diameter @ fall 1:160 = 0.625% OK



Surface Water Design

Pipe Coefficient k=0.06 (new PVC Pipe), Vmin=0.8m/s, Vmax=4.0m/s, V(ideal)=1.8m/s

Area for Road Section: from B to C
 Manholes: from S7 to S8

Distance between Manholes = 21.159 m

Road	=	143.11 m	x	1.0 m	=	143.11 m ²	x	100.00% (impermeability)	=	143.11 m ²
Footpath & Paving	=	41.20 m	x	1.0 m	=	41.20 m ²	x	100.00% (impermeability)	=	41.2 m ²
Car Park Spaces	=	0.00 m ²	x	1.0 No	=	0.00 m ²	x	100.00% (impermeability)	=	0 m ²
Driveway	=	0.00 m ²	x	0.0 No	=	0.00 m ²	x	100.00% (impermeability)	=	0 m ²
Area of Roofs	A	0.00 m ²	x	1.0 No	=	0.00 m ²	x	100.00% (impermeability)	=	0 m ²
Area of Roofs		0.00 m ²	x	0.0 No	=	0.00 m ²	x	100.00% (impermeability)	=	0 m ²
Green Area		0.00 m ²	x	1.0 No	=	0.00 m ²	x	10.00% (impermeability)	=	0 m ²
Grass Verge	0	56.25 m ²	x	1.0 No	=	56.25 m ²	x	10.00% (impermeability)	=	5.625 m ²

Total 184.31 m²

Taking Rain Intensity of 50 mm per Hour

$$\text{Vol of Water try} = \frac{184.31 \text{ m}^2}{225 \text{ diameter pipe at fall}} \times 0.05 \text{ m} = 9.22 \text{ m}^3/\text{hr} = 2.56 \text{ l/s}$$

Therefore $v = 0.76 \text{ m/s} = 2.56 \text{ L/S}$

Total Area = 184.31 m² = 0.05 Acres

$$Q = 60.5 \times A_p \times R$$

Where A_p = Impermeable Area (Acres)
 R = Rate of Discharge (inches).

Time Taken for Surface Water to travel between Manholes = $\frac{21.159}{0.76} = 27.84 = 0.46 \text{ mins}$

Time of Concentration (T) = 0.46 + 3.0 = 3.46 min

$$R = \frac{30}{10 + T} = \frac{30}{10 + 3.4601316} = 2.23$$

Q(actual) = 60.5 x 0.04554 acres x 2.23 inches = 6.14 ft³/min = 2.90 l/s = 2.90L/S

Therefore use 225 diameter @ fall 1:100 = 1.000% OK



Surface Water Design

Pipe Coefficient k=0.06 (new PVC Pipe), Vmin=0.8m/s, Vmax=4.0m/s, V(ideal)=1.8m/s

Area for Road Section: from Manholes from C to S8 to S9 D to E to S4

Distance between Manholes	=	MAX	27.009 m							
Road	=	290.38 m	x	1.0 m	=	290.38 m ²	x	95.00% (impermeability)	=	275.861 m ²
Foolpath	=	45.20 m	x	2.0 m	=	90.40 m ²	x	95.00% (impermeability)	=	85.88 m ²
Car park	=	0.00 m ²	x	1.0 No	=	0.00 m ²	x	100.00% (impermeability)	=	0 m ²
Driveway	=	0.00 m ²	x	0.0 No	=	0.00 m ²	x	100.00% (impermeability)	=	0 m ²
Area of Roofs	=	58.54 m ²	x	4.0 No	=	234.16 m ²	x	100.00% (impermeability)	=	234.16 m ²
Area of Roofs	=	0.00 m ²	x	0.0 No	=	0.00 m ²	x	100.00% (impermeability)	=	0 m ²
Grass Verge	=	19.90 m ²	x	1.0 No	=	19.90 m ²	x	10.00% (impermeability)	=	1.99 m ²
Green Area	=	102.58 m ²	x	1.0 No	=	102.58 m ²	x	10.00% (impermeability)	=	10.258 m ²
Total									=	608.15 m²

Taking Rain Intensity of 50 mm per Hour

Vol of Water = 608.15 m² x 0.05 m = 30.41 m³/hr = 8.45 l/s
 try 225 diameter pipe at fall 1:160 Therefore v= 0.9 m/s "+" 2.90l/s = 11.35L/s

Total Area = 608.15 m² = 0.15 Acres

$Q = 60.5 \times A_p \times R$

Where A_p = Impermeable Area (Acres)
 R = Rate of Discharge (inches).

Time Taken for Surface Water to travel between Manholes = $\frac{27.009}{0.9} = 30.01$ = 0.50 mins

Time of Concentration (T) = 0.50 + 3.0 = 3.50 min

$R = \frac{30}{10 + T} = \frac{30}{10 + 3.50016667} = 2.22$

Q(actual) = 60.5 x 0.15027 acres x 2.22 inches = 20.20 ft³/min = 9.53 l/s + 2.90l/s

Therefore use 225 diameter @ fall 1:160 = 0.625% OK = 12.43L/s

TOTAL IMPERMEABLE AREA = 792.46 Sq.m



STORMTECH Stormwater Management System Design Tool

PROJECT REF:	St. Finian's Way Newcastle, Co. Dublin
LOCATION:	Tank 1
DATE:	1st April 2019
CREATED BY:	HOR

SYSTEM PARAMETERS

Required Total Storage	234.42 m ³
Stormtech chamber model	MC3500
Filtration Permeable Geo or Impermeable Geo	Filter geo
Number of Isolator Rows (IR)	1

SITE PARAMETERS

Stone Porosity	40%	
Excavation Batter Angle (degrees)	60	<i>Minimum Requirement</i>
Stone Above Chambers	0.3 m	0.30
Stone Below Chambers	0.23 m	0.23
In-between Row Spacing	0.23 m	0.23
Additional Storage outside Excavation. E.g manholes. Header Pipe	0 m ³	

HEADER PIPE

Is Header pipe required within excavation	No
Orientation of Header Pipe	Parallel to IR
Diameter of Header Pipe	0.6 m
Length of Header Pipe	0 m

CHAMBER SYSTEM DIMENSIONS

	Calculated	Adopted
Number of Rows		4 ea
Number of units per Row		10 ea
System Installed Storage Depth (effective storage depth)	1.675	m
Tank overall installed Width at base	9.11	9.11 m
Tank overall installed Length at Base	23.54	23.6 m
Total Effective System Storage	243.6	244.0 m³

STORMTECH SYSTEM D

StormTech Chamber Mod
Unit Width
Unit Length
Unit Height
Min Cover Over System
Max Cover Over Chamber
Chamber Internal Storage
Header Pipe Internal Storage

STONE AND EXCAVATIC

Volume of Dig for System
Width at base
Width at top
Length at base
Length at top
Depth Of System
Area of Dig at Base of Sys
Area of Dig at Top of Syste
Void Ratio
Stone Requirement - m3
Stone Requirement - tonne

STORMTECH Stormwater Management System Design Tool

PROJECT REF:	St. Finian's Way Newcastle Co. Dublin
LOCATION:	Attenuation Tank
DATE:	7th February 2022
CREATED BY:	HOR

SYSTEM PARAMETERS

Required Total Storage	102 m ³
Stormtech chamber model	SC740
Filtration Permeable Geo or Impermeable Geo	Filter geo
Number of Isolator Rows (IR)	1

SITE PARAMETERS

Stone Porosity	40%	
Excavation Batter Angle (degrees)	60°	<i>Minimum Requirement</i>
Stone Above Chambers	0.15 m	0.15
Stone Below Chambers	0.15 m	0.15
In-between Row Spacing	0.15 m	0.15
Additional Storage outside Excavation. E.g manholes. Header Pipe	0 m ³	

HEADER PIPE

Is Header pipe required within excavation	No
Orientation of Header Pipe	Parallel to IR
Diameter of Header Pipe	0.6 m
Length of Header Pipe	0 m

CHAMBER SYSTEM DIMENSIONS

	Calculated	Adopted
Number of Rows		3 ea
Number of units per Row		14 ea
System Installed Storage Depth (effective storage depth)	1.060	m
Tank overall installed Width at base	4.79	4.8 m
Tank overall installed Length at Base	31.08	31.1 m
Total Effective System Storage	105.7	105.9 m³

STORMTECH SYSTEM D

StormTech Chamber Mod
Unit Width
Unit Length
Unit Height
Min Cover Over System
Max Cover Over Chamber
Chamber Internal Storage
Header Pipe Internal Storage

STONE AND EXCAVATIC

Volume of Dig for System
Width at base
Width at top
Length at base
Length at top
Depth Of System
Area of Dig at Base of Sys
Area of Dig at Top of Syste
Void Ratio
Stone Requirement - m3
Stone Requirement - tonne

Thor Design & Management.

Structural, Mechanical, Surveying
& Building Services Engineering Consultancy

Attenuation Tank 1

Attenuation Calculations - Overflow

Development - Additional 6No. Dwellings @ St. Finian's Way, Newcastle, Co. Dublin

Calculation of storage volumes at surface water outfall

Area of Development (acres)	1.771 Acres
Area of Development: (hectares)	0.72 Ha
Impermeable Area (hectares) =	0.3786 Ha
Total Allowable Outflow	7.00 litres/second

Storm Duration (mins) Rainfall (mm)

15	27.8
30	34.8
60	43.7
120	54.7
240	68.5
360	78.2
720	98

Storm return period of 1 in 100 years

Storm Duration (mins)	Rainfall (mm)	Rainfall Intensity (mm/hr)	Flow Rate (l/s)	Excess Flow Above Outflow
15	33.36	133.44	153.20	146.20
30	41.76	83.52	95.89	88.89
60	52.44	52.44	60.20	53.20
120	65.64	32.82	37.68	30.68
240	82.2	20.55	23.59	16.59
360	93.84	15.64	17.96	10.96
720	117.6	9.80	11.25	4.25

Therefore, storage volume required =

238.94 m3



Thor Design & Management.

Structural, Mechanical, Surveying
& Building Services Engineering Consultancy

Attenuation Tank 2

Attenuation Calculations - Overflow

Development - Additional 6 No. Dwelling @ St. Finian's Way, Newcastle, Co. Dublin

Calculation of storage volumes at surface water outfall

Area of Development (acres)	1.771 Acres
Area of Development (hectares)	0.72 Ha
Impermeable Area (hectares) =	0.0792 Ha
Total Allowable Outflow	7.00 litres/second

Storm Duration (mins) Rainfall (mm)

15	27.8
30	34.8
60	43.7
120	54.7
240	68.5
360	78.2
720	98

Storm return period of 1 in 100 years

Storm Duration (mins)	Rainfall (mm)	Rainfall Intensity (mm/hr)	Flow Rate (l/s)	Excess Flow Above Outflow
15	33.36	133.44	39.05	32.05
30	41.76	83.52	27.06	20.06
60	52.44	52.44	19.59	12.59
120	65.64	32.82	14.88	7.88
240	82.2	20.55	11.94	4.94
360	93.84	15.64	10.76	3.76
720	117.6	9.80	9.35	2.35

7Ls Included from Attenuation Tank

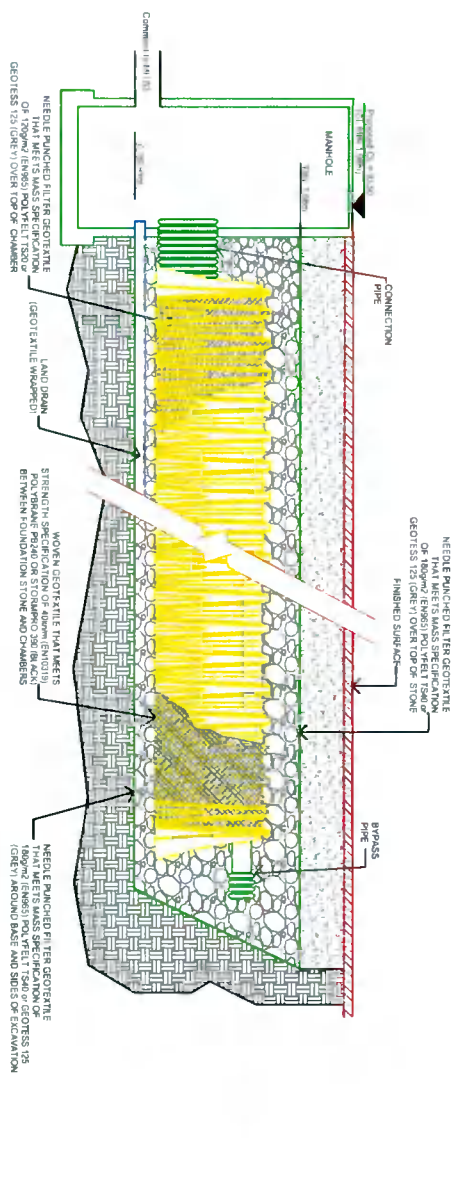
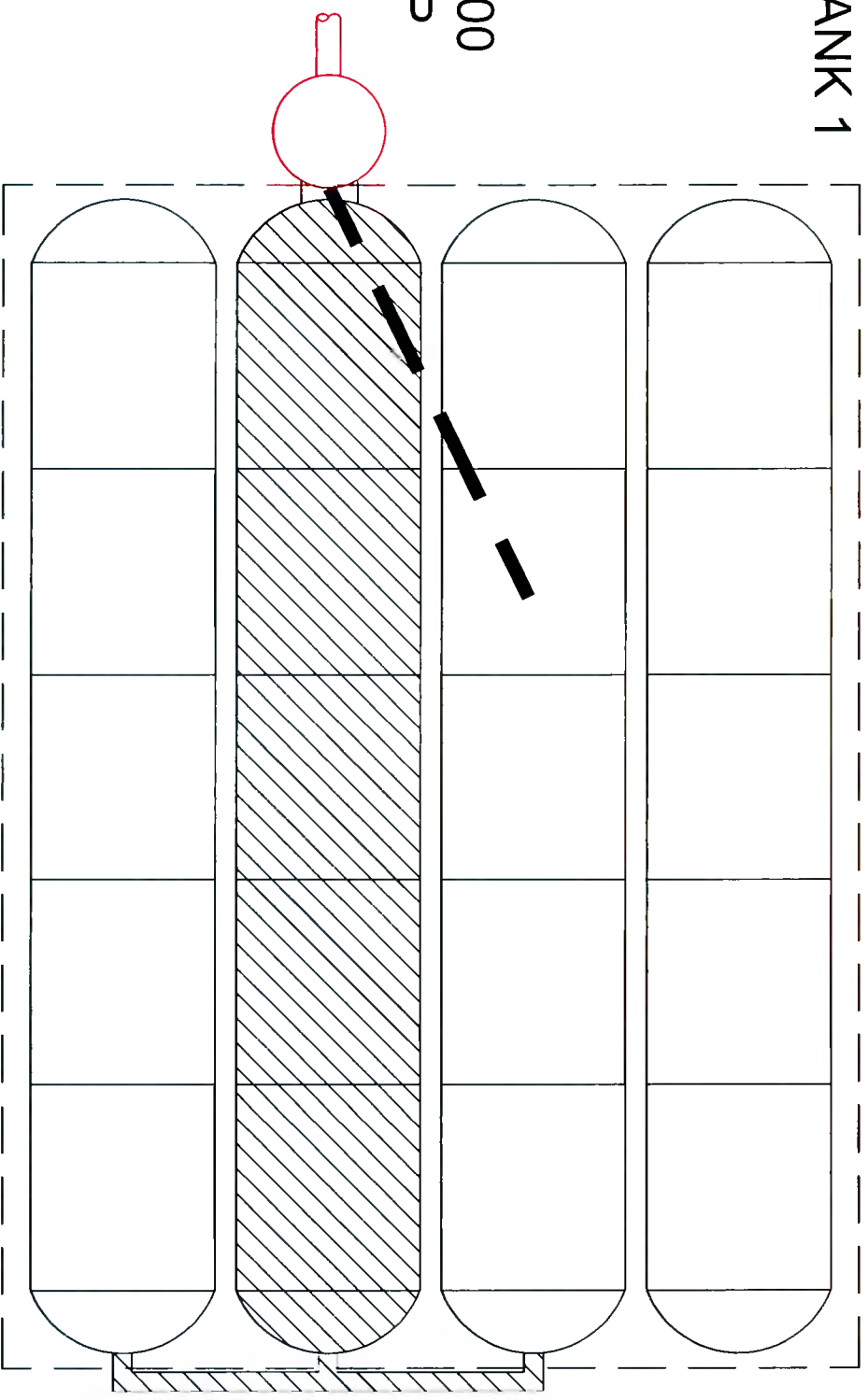
Therefore, storage volume required =

101.68 m³



ATTENUATION TANK 1

OFFLINE MC3500 SYSTEM SETUP

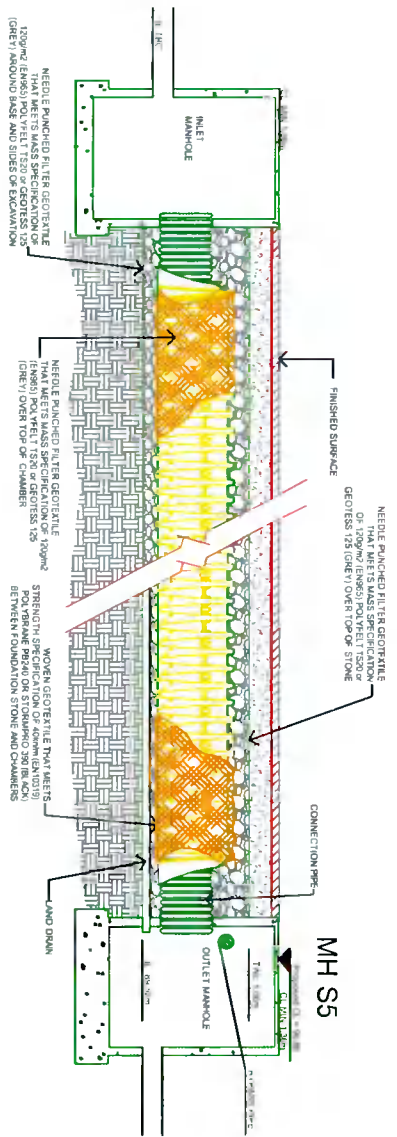
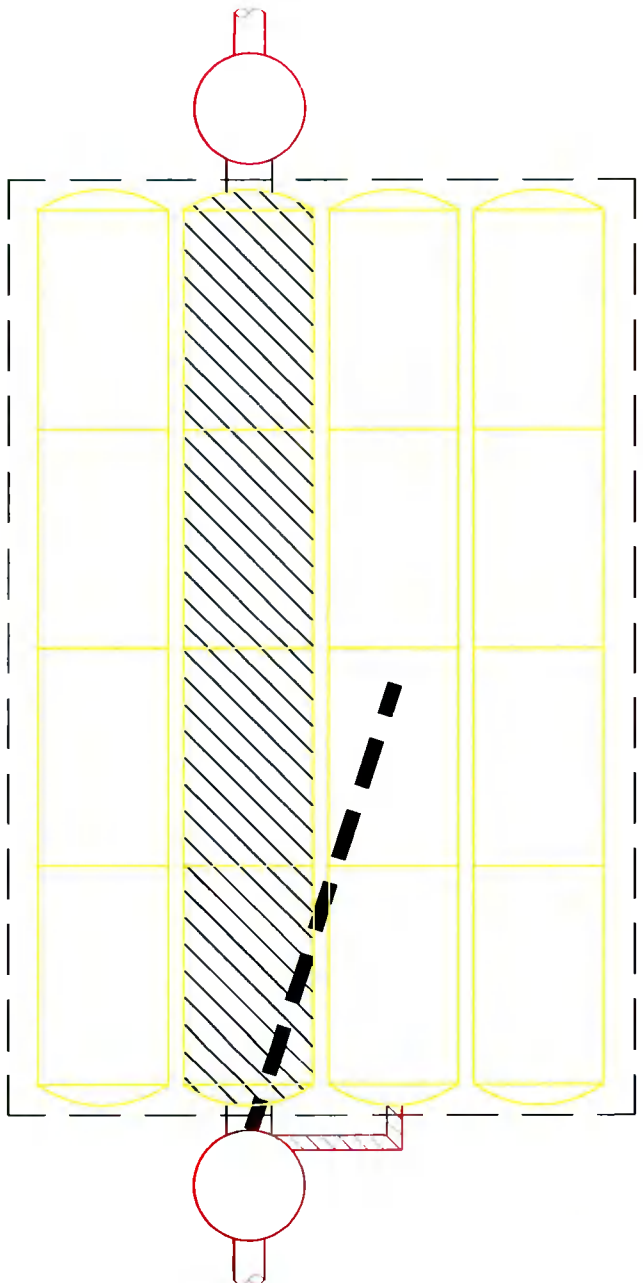


Proposed Additional Info: Drawings St. Eunan's Way, Newcastle, Co. Dublin Powerent Homes Ltd Site Drainage	PH-225-03 P1.1 7th Feb 2022
EXISTING STORMTECH ATTENUATION SYSTEM	P1.1



ATTENUATION TANK 2

ONLINE SC740 SYSTEM SETUP



NEEDLE PUNCHED FILTER GEOTEXTILE THAT MEETS MASS SPECIFICATION OF 125 G/SM OVER TOP OF STONE

NEEDLE PUNCHED FILTER GEOTEXTILE THAT MEETS MASS SPECIFICATION OF 125 G/SM OVER TOP OF CHAMMER

WOVEN GEOTEXTILE THAT MEETS STRENGTH SPECIFICATION OF 400N/1000G BETWEEN FOUNDATION STONE AND CHAMMERS



01 ATTENUATION TANK 02 FILTERING 03 DRAINAGE 04 EXISTING STORMWATER ATTENUATION SYSTEM	11-24-2021 Approved 15
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Proposed Additional Site Drawings St. Edmond's Way, Newcastle, Co. Dublin Powerpoint Homes Ltd Site Drainage PHL-225-08 7th Feb. 2022	P1 19
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No. **KEL-CPR-010**

1. Unique identification code of the product-type:

**Separator Systems for Light Liquids, Plastic Construction
NSBP003, NSBP004 & NSBP006**

2. Type, batch or serial number or any other element allowing identification of the construction product as required under Article 11(4) of the CPR:

**Serial Number/Works Order Number printed on the Product Information Label
& affixed to product**

3. Intended use or uses of the construction product, in accordance with the applicable harmonized technical specification, as foreseen by the manufacturer:

Collection & Separation of Light Liquids from Waste Water by means of gravity and/or coalescence

4. Name, registered trade name or registered trade mark and contact address of the manufacturer as required under Article 11(5):

**Kingspan Environmental Ltd
College Rd North
Aston Clinton, Aylesbury, Buckinghamshire
HP22 5EW**

5. Where applicable, name and contact address of the authorised representative whose mandate covers the tasks specified in Article 12(2):

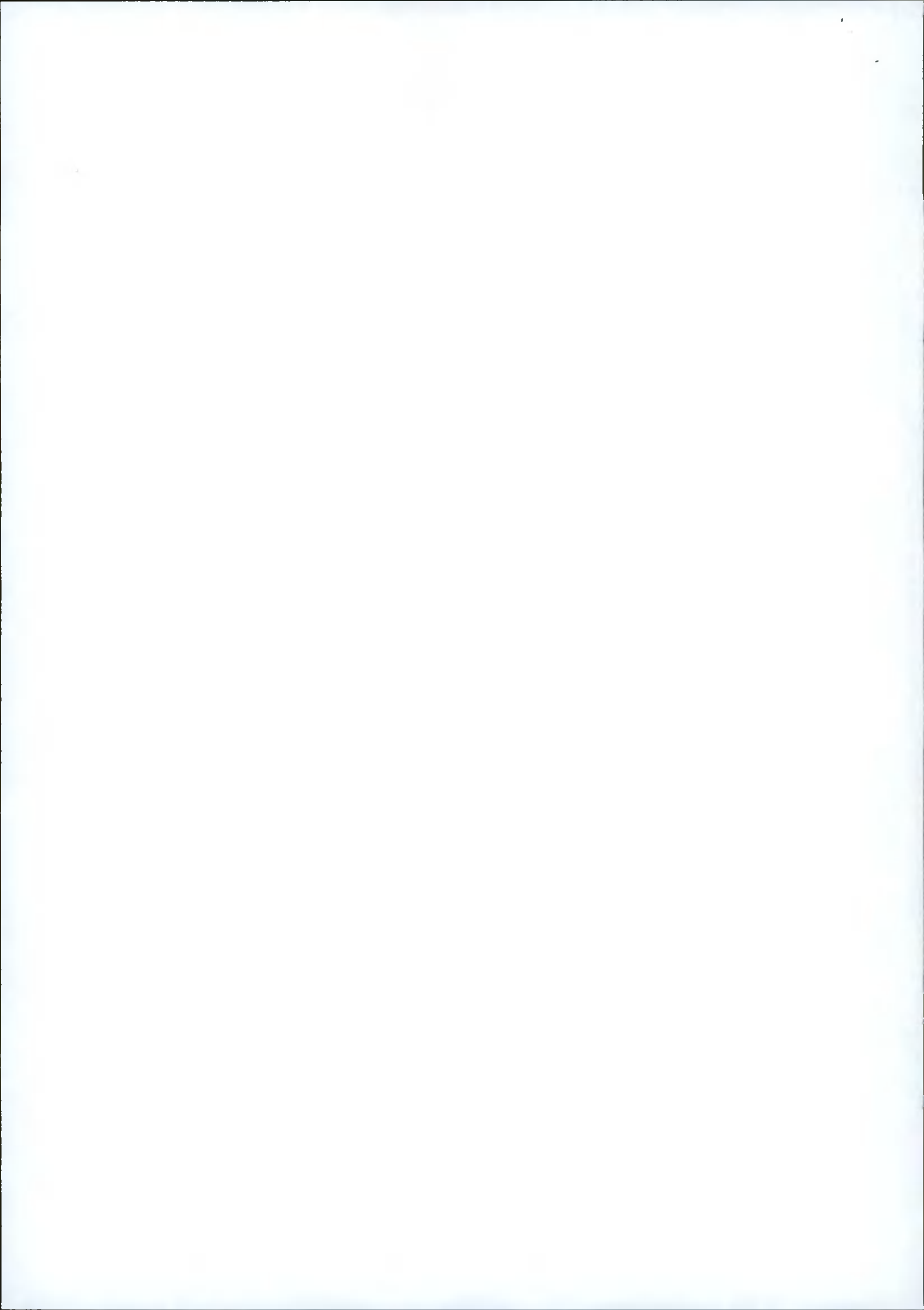
N/A

6. System or systems of assessment and verification of constancy of performance of the construction product as set out in CPR, Annex V:

System 3

7. In case of the declaration of performance concerning a construction product covered by a harmonized standard:

**EN 858-1:2002
BSI, Maylands Avenue, Hemel Hempstead, Herts HP2 4SQ
Has executed initial type testing according to system 3 and delivered the test report**



8. Declared performance:

Essential Characteristics		Performance			Harmonised technical specification
Crushing Resistance (vertical load test)		Pass (also wet conditions)			EN 858-1:2002
Structural Behaviour		Pass			
Resistance to fire		Class E			
Water Tightness (water test)		Pass			
Material Durability		MFR (190/2,16) = 3.0± 1g/10 min (ISO 1133)			
		Density ≥ 939 kg/m ³ (ISO 1872)			
		Yield Stress ≥ 19 Mpa (ISO 527-2)			
		Pressure A1 = 1 (EN1778)			
		Pressure A2K = 1 (EN1778)			
Treatment Efficiency	Sample	Specified Maximum Light Liquid (mg/l)	Actual Light Liquid (mg/l)		
	1	≤10	0.37	Pass	
	2	≤10	0.22	Pass	
	3	≤10	0.35	Pass	
	4	≤10	0.23	Pass	
	5	≤10	0.35	Pass	
	Average	≤5	0.30	Pass	
Electrical Consumption		n/a			

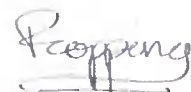
Signed for and on behalf of the manufacturer by:

Paul Copping – Technical Director

.....
(Name and function)

Aylesbury – 13th May 2013

.....
(Place and date of issue)



.....
(Signature)

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