

BYE LAW APPLICATION FEES



REF. NO.: 92A/0047 CERTIFICATE NO.: 17231 B

PROPOSAL: Internal office Development

LOCATION: Unit 3 Broomhill Terrace, Tallaght Industrial Estate

APPLICANT: Artech Ireland

Key 15/7/91

	1	2	3	4	5	6	7
CLASS	DWELLINGS/AREA LENGTH/STRUCTURE	RATE	AMT. OF FEE REQUIRED	AMT. (ADDED)	BALANCE DUE	RED. FEE APPL.	AMT. OF RED. FEE
A	Dwelling (Houses/Flats)	@ £55					
B	Domestic Ext. (Improvement/Alts.)	@ £30					
C	Building for office or other comm. purpose	@ £3.50 per M ² or £70		£70			
D	Building or other structure for purposes of agriculture	@ £1.00 per M ² in excess of 300 M ² Min. £70					
E	Petrol Filling Station	£200					
F	Dev. of prop. not coming within any of the forgoing classes	£70 or £9 per .1 hect. whichever is the greater	£70		Paid in full. NIC £70 £70 transferred fees		

Many thanks OR 27671

Column 1 Certified: Signed: _____ Grade: _____ Date: _____

Column 1 Endorsed: Signed: _____ Grade: _____ Date: _____

Columns 2,3,4,5,6 & 7 Certified: Signed: RWA Grade: S.O Date: 17/1/92

Columns 2,3,4,5,6 & 7 Endorsed: Signed: _____ Grade: _____ Date: _____

LOCAL GOVERNMENT (PLANNING AND DEVELOPMENT) ACTS, 1963 TO 1982

ASSESSMENT OF FINANCIAL CONTRIBUTION

REG. REF.: 02A/047

CONT. REG.:

SERVICES INVOLVED: WATER/FOUL SEWER SURFACE WATER

AREA OF SITE:

FLOOR AREA OF PRESENT PROPOSAL: 3553 ^{FT²}
J. Y.
17/1/92.

MEASURED BY:

CHECKED BY:

METHOD OF ASSESSMENT:

TOTAL ASSESSMENT:

MANAGER'S ORDER NO: P / / DATED

ENTERED IN CONTRIBUTIONS REGISTER:

DEVELOPMENT CONTROL ASSISTANT GRADE

PLANNING APPLICATION FEES

Reg. Ref. 92A/0047

Cert. No. 21671

PROPOSAL Internal office development

LOCATION Unit 3, Beccanhill Terrace, Tallaght Industrial Estate

APPLICANT Aratech Ireland

CLASS	DWELLINGS/AREA LENGTH/STRUCT.	RATE	AMT. OF FEE REQ.	AMOUNT LODGED	BALANCE DUE	BALANCE PAID
1	Dwellings	@£32				
2	Domestic	@£16				
3	Agriculture	@50p per m2 in excess of 300m2. Min. £40				
4	Metres 330.0m	@£1.75 per m2 or £40	£ 577.50	647.50	£70 overpayment	
5	x .1 hect.	@£25 per .1 hect. or £250				
6	x .1 hect.	@£25 per .1 hect. or £40				
7	x .1 hect.	@£25 per .1 hect. or £100				
8		@£100				
9	x metres	@£10 per m2 or £40				
10	x 1,000m	@£25 per £1000m or £40				
11	x .1 hect.	@£5 per .1 hect. or £40				

Column 1 Certified: Signed: *[Signature]* Grade: *D/PL* Date: *17/1/92*

Column 1 Endorsed: Signed: Grade: Date:

Columns 2,3,4,5,6 & 7 Certified: Signed: *[Signature]* Grade: *S.O* Date: *17/1/92*

Columns 2,3,4,5,6 & 7 Endorsed: Signed: Grade: Date:

Geraldine Boothman

SS only

Ⓜ

Register Reference : 92A/0047

Date : 21st January 1992

Development : Internal office development and ancillary services

LOCATION : Unit 3, Broomhill Terrace, Tallaght Industrial Estate
Co. Dublin

Applicant : Aritech Ireland

App. Type : PERMISSION/BUILDING BYE-LAW APPROVAL

Planning Officer : G. BOOTHMAN

Date Recd. : 15th January 1992

Attached is a copy of the application for the above development .Your report would be appreciated within the next 28 days.

Yours faithfully,

DUBLIN Co. COUNCIL
27 JAN 1992
SAN SERVICES

.....
for PRINCIPAL OFFICER

Date received in Sanitary Services

FOUL SEWER

*Available.
Any effluent other than domestic effluent to be subject to the provisions of the Water Pollution Act.*

PLANNING DEPT.
DEVELOPMENT CONTROL
Date 23. 03. 92
Time 11. 00

SURFACE WATER

*Available.
Surface water run-off to be subject to the provisions of the Water Pollution Act.*

SENIOR ENGINEER,
SANITARY SERVICES DEPARTMENT,
46/49 UPPER O'CONNELL STREET,
DUBLIN 1

J.P. 6/3/92
6/3/92

Register Reference : 92A/0047

Date : 21st January 1992

.....
ENDORSED _____ DATE _____

WATER SUPPLY... NOT APPLICABLE

R. J. Spain
19/2/92

J. J. H. H. H. H.
17/2/92

Refer to C.F.O.

.....
ENDORSED *[Signature]* DATE 2/3/92

PLANNING DEPT.
DEVELOPMENT CONTROL
Date 23.03.92
Time 11.00

P/934/92

ON 834 88 1150

COMHAIRLE CHONTAE ÁTHA CLIATH

Record of Executive Business and Manager's Orders

BELGARD

CONTRIBUTION:	
Standard:	All
Formal:	in
S.S.:	full
Other:	in
SECURITY:	in
Donor:	O.F.E.
Date:	

Register Reference : 92A/0047 Date Received : 15th January 1992

Correspondence : Dexion Ireland Limited,
Name and : Unit 15, Clondalkin Industrial
Address : Estate,
Clondalkin,
Dublin 22

Development : Internal office development and ancillary services

Location : Unit 3, Broomhill Terrace, Tallaght Industrial Estate
Co. Dublin

Applicant : Aritech Ireland

App. Type : Permission

Zoning : E "to provide for industry and related uses"

Floor Area : 370 Sq.metres

(RO'D/CM)

Report of the Dublin Planning Officer, dated 28th February, 1992.

This is an application for Permission for internal office development and ancillary services at Unit 3, Broomhill Terrace, Tallaght Industrial Estate, for Aritech Ireland Ltd.

The site is zoned 'E' - "to provide for industry and related uses" in the 1983 Development Plan.

PLANNING HISTORY:

Reg. Ref. 89A/456: Permission was granted for 4 no. factory or warehouse units with associated offices at Broomhill Road, Tallaght, for Streamstown Investments Ltd. on 11th May, 1989.

The current application proposes to convert 370sq. metres of warehouse space into Administration Offices. This involves the installation of ground floor and mezzanine level partitioning in the existing warehouse area. A suspended ceiling is proposed over the entire area and underneath the mezzanine level. On the upper mezzanine two offices are proposed with the remainder located at ground floor. The existing stores area which is approximately ~~1000sq. ft.~~ ^{330 m²} is to be retained.

The applicant, Aritech Ireland Ltd. is the largest manufacturer of Burglar

COMHAIRLE CHONTAE ÁTHA CLIATH

Record of Executive Business and Manager's Orders

Reg.Ref: 92A/0047

Page No: 0002

Location: Unit 3, Broomhill Terrace, Tallaght Industrial Estate Co.
Dublin

Alarm Equipment in the country. The firm employs 135 people in the Tallaght area and operates out of three buildings, two of which are located in Broomhill Drive while the third, the subject of the current application, is located in Broomhill Terrace. The factory and warehousing units are based in Broomhill Drive while the distribution and storage operation is conducted from the current site.

The applicant has been operating out of the site premises for approximately 4 years. During this period the building has been used for office and storage purposes to facilitate the distribution aspect of the business. The applicant now wishes to officially regularise this situation.

Office development is not normally permissible in an industrial zone. However, given the particular nature of the applicants business, i.e. three inter-related units operating in close proximity to each other at Broomhill Estate, the size of the offices proposed and the significant area of storage which is to be retained in the building,

I recommend that a decision to Grant Permission be made under the Local Government (Planning and Development) Acts, 1963-1990, subject to the following (3) conditions:-

CONDITIONS / REASONS

01 The development to be carried out in its entirety in accordance with the plans, particulars and specifications lodged with the application save as may be required by the other conditions attached hereto.

REASON: To ensure that the development shall be in accordance with the permission and that effective control be maintained.

02 That before development commences, approval under the Building Bye-Laws be obtained and all conditions of that approval be observed in the development.

REASON: In order to comply with the Sanitary Services Acts, 1878-1964.

03 That the requirements of the Chief Fire Officer be ascertained and strictly adhered to in the development.

REASON: In the interest of safety and the avoidance of fire hazard.

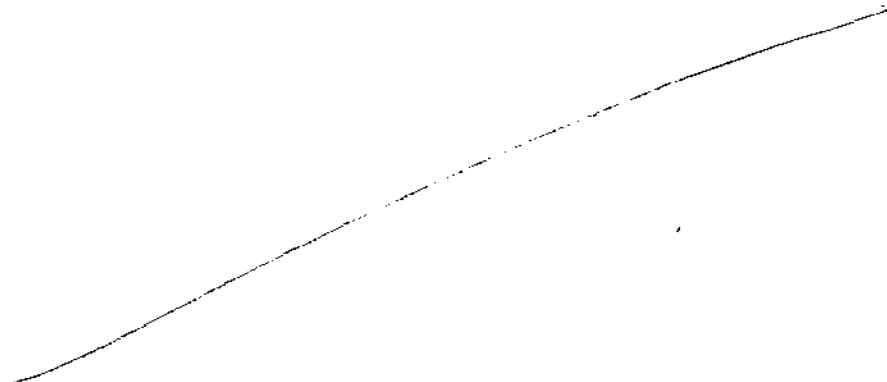
COMHAIRLE CHONTAE ÁTHA CLIATH

Record of Executive Business and Manager's Orders

Reg.Ref: 92A/0047

Page No: 0003

Location: Unit 3, Broomhill Terrace, Tallaght Industrial Estate Co.
Dublin



Richard Cronin SEP
.....
for Dublin Planning Officer

28/2/92

Endorsed: *for Dase*
.....
for Principal Officer

Order: A decision pursuant to Section 26(1) of the Local Government (Planning and Development) Acts, 1963-1990 to GRANT PERMISSION for the above proposal subject to the () conditions set out above is hereby made.

Dated : *5th March*
.....
FEBRUARY 1992

John [Signature]
.....
ASSISTANT COUNTY MANAGER/APPROVED OFFICER

to whom the appropriate powers have been delegated by order of the Dublin City and County Manager dated *18th February* 1992.

Reg.Ref. 92A/0047
Decision Order No. P/ 0934 /91
Page No: 0002



Bloc 2, Ionad Bheatha na hEireann,
Bloc 2, Irish Life Centre,
Sraid na Mainistreach Iacht,
Lower Abbey Street,
Baile Atha Cliath 1.
Dublin 1.
Telephone (01) 724755
Fax (01) 724896

C O N D I T I O N S / R E A S O N S

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CI/Sfb	22.3	Xh2

JUNE 1990

MASTER RANGE



L&D PARTITIONING SYSTEMS LTD



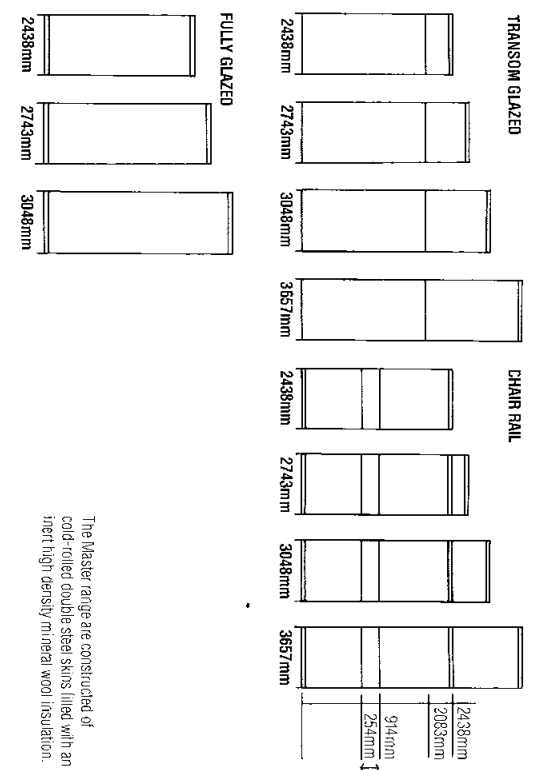
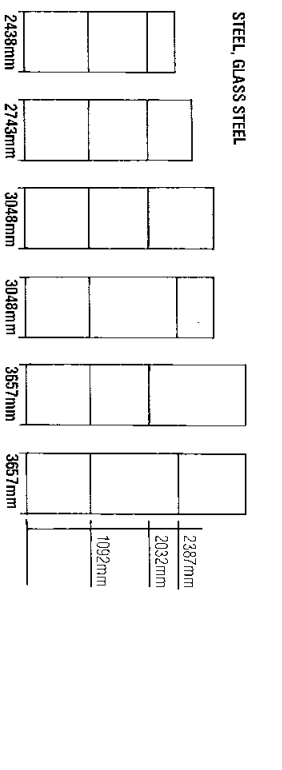
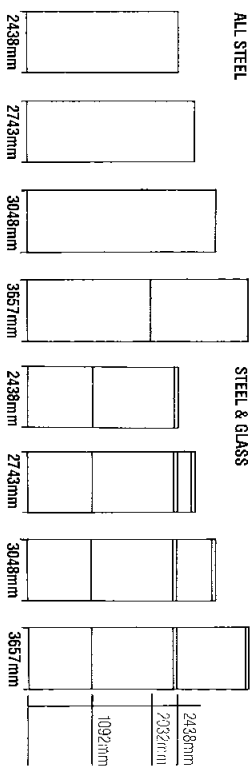
MASTER PARTITIONING SYSTEM

MASTER DOUBLE SKIN STEEL PANEL RANGE

INTRODUCTION

L & D Partitioning Systems Ltd. is one of the most established and successful manufacturers of steel monobloc partitioning in the UK. The name of L & D has long been associated with the virtues of quality and unparalleled service. These strengths have been enhanced further as the Company enters the opportunities of the 1990's. Rapidly expanding into Europe and worldwide markets, L & D guarantee that all product ranges are manufactured

to quality assured procedures to ISO 9002 BS 5750 pt 2 1987. The Master Range is L & D's most popular steel monobloc partitioning system. Fully demountable and relocatable the insulated double skin steel panels offer a most comprehensive range of module variations to create a cost-effective and versatile system ideally suited for the working office environment.



The Master range are constructed of cold-rolled double steel skins filled with an inert high density mineral wool insulation.



MASTER PARTITIONING SYSTEM

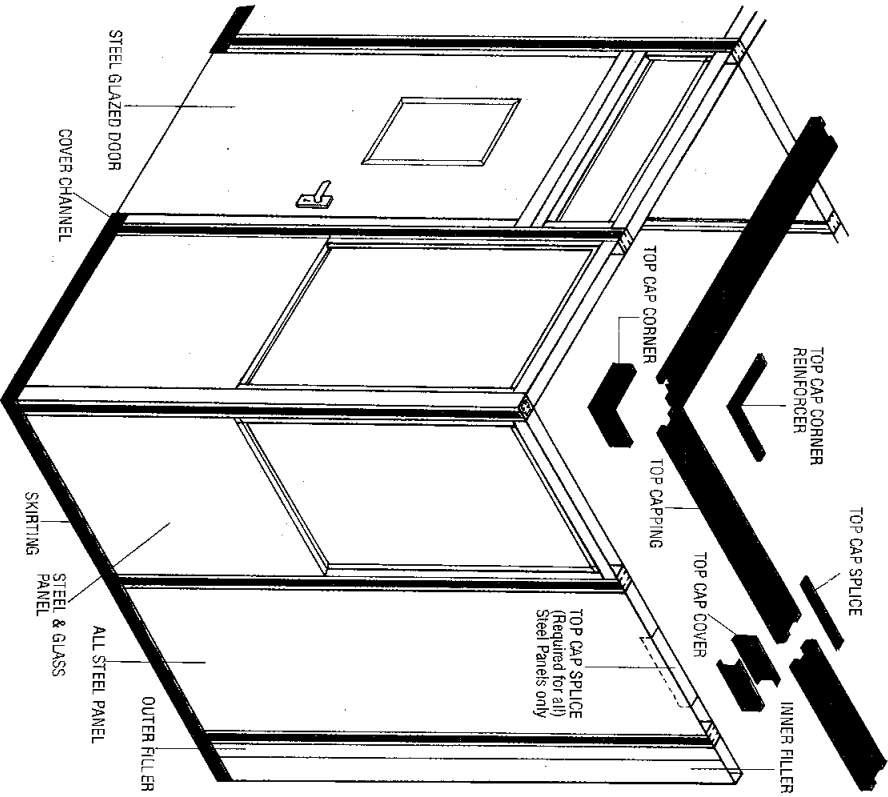
COMPOSITION AND MANUFACTURE

L & D's Master range of fully relocatable partitioning is available in solid and fully or part glazed elevations, and can either be ceiling fixed or free standing. The partitioning is manufactured from cold rolled steel sections, supporting two surface pressed steel panels, and the cavity between the panels is filled with inert high density mineral wool insulation.

The panels are finished with an electrostatically applied epoxy powder coating.

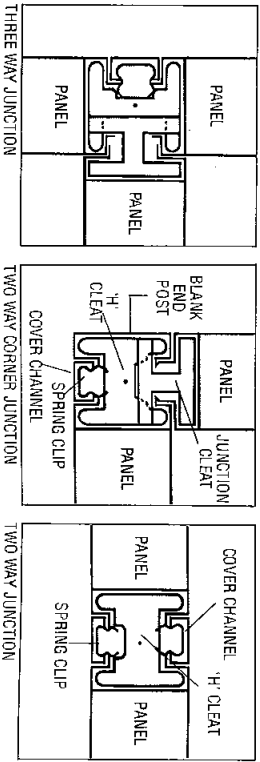
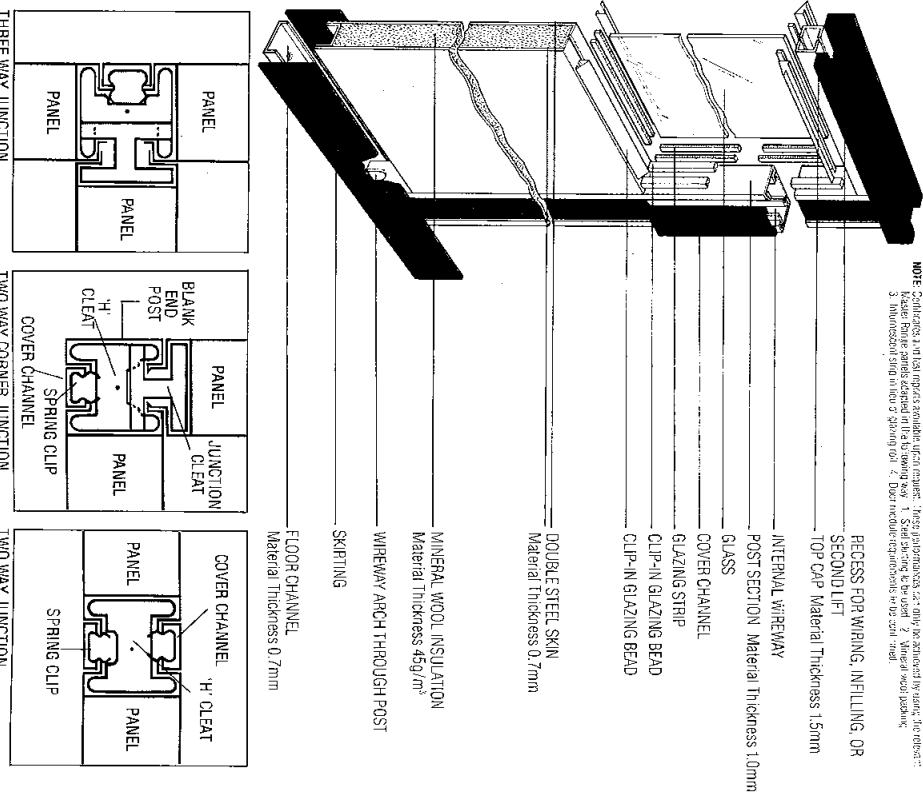
The system is located and housed in a channel which is secured to the floor, and vertical panel posts providing rigidity and strength are securely locked together with specially designed cleats. The vertical posts

extend below the panel base and are constructed in such a way as to provide the facility for a complete wirework management system both horizontally and vertically via the post sections. Detachable skirting and cover channels over the vertical posts provide easy access to wiretrays, and wiring can also be incorporated in the top capping of the system.



FIRE CERTIFICATION/PERFORMANCE					
TYPE	RATING	TYPE	RATING	TYPE	RATING
All Steel Elevation with 140 Steel Skirting	BS 576 P1/6 30 mins. Integ. 12 Insul.	All Steel Elevation with 140 Steel Skirting	BS 576 P1/22 30 mins. Integ. 12 Insul.	All Steel Elevation with 140 Steel Skirting	BS 576 P1/22 30 mins. Integ. 12 Insul.
All Steel Elevation with 220 Steel Skirting	BS 576 P1/22 100 mins. Integ. 12 Insul.	All Steel Elevation with 220 Steel Skirting	BS 576 P1/22 100 mins. Integ. 12 Insul.	All Steel Elevation with 220 Steel Skirting	BS 576 P1/22 100 mins. Integ. 12 Insul.
Steel Glazed Steel Elevation with 100 Steel Skirting	BS 576 P1/22 51 mins. Integ. 5 Insul.	Steel Glazed Steel Elevation with 100 Steel Skirting	BS 576 P1/22 51 mins. Integ. 5 Insul.	Steel Glazed Steel Elevation with 100 Steel Skirting	BS 576 P1/22 51 mins. Integ. 5 Insul.

NOTE: Conditions of test are as per BS 576-1:1982. For details of construction, refer to the relevant BS 576-1:1982. For details of construction, refer to the relevant BS 576-1:1982. For details of construction, refer to the relevant BS 576-1:1982.



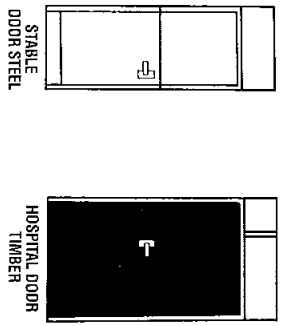
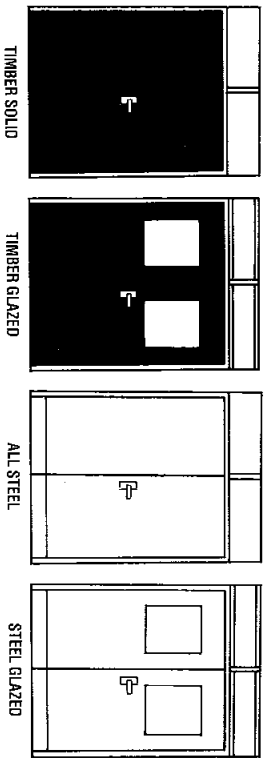
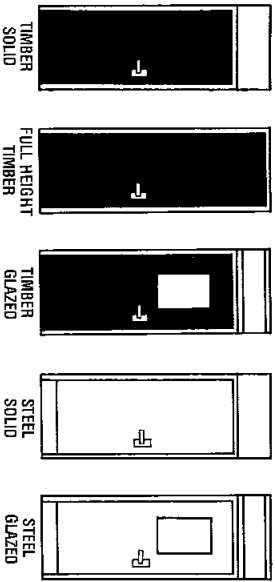
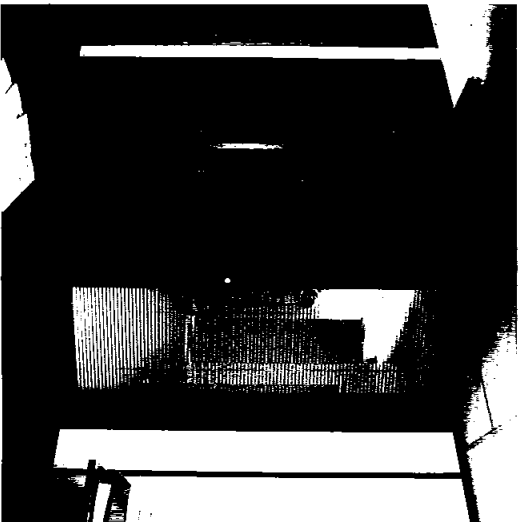


DOORS & ACCESSORIES

DOORS & ACCESSORIES

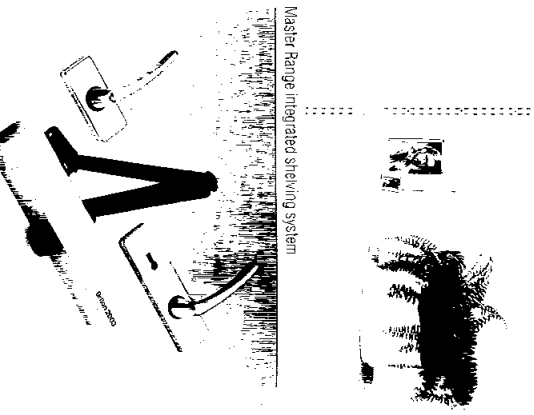
Versatile options within the Master range include full height doors constructed of double-skin steel, cellular or solid timber, and doors with matching overpanels to give a distinctive finishing touch. Good quality standard door furniture is provided as standard, and special level handles and knockers are available to suit special requirements.

Easily accessible internal airways are provided within the post sections, skirtings and heads for power and communication cables and built-in shelving systems to support various items of office equipment, are available. Cut-outs for light switches, sockets, gills and air vents can be provided to order.



N.B. Door configurations shown above are only a small selection of designs available. Choice of material options for doors and overpanels or any other special requirements are available upon request.

Master Range integrated shelving system



TECHNICAL SPECIFICATION

STEEL

Master range of partitioning is manufactured from high grade steel — BS1449 part 1:1972 grade CR4 or equivalent. Thickness of steel used in manufacture: vertical posts 1.0mm; panel skins 0.7mm; floor channel 0.7mm; top capping 1.5mm.

MODULES

Modules are constructed in standard widths decreasing from 900mm to 300mm in 100mm increments, and adjustable wall fillers are supplied to suit individual layouts. Special optimal module widths in increments of 1mm are available to order.

HEIGHT

Standard nominal heights are 2438mm, 2743mm, 3048mm and 3657mm. Purpose built panels can also be manufactured to suit individual requirements, and the system can be multi-tiered.

THICKNESS

Nominally 50mm.

INSULATION MATERIAL

Cavity between panels filled with inert high density mineral wool 45g/m³

FINISH

Electrostatically applied epoxy powder coated to a thickness of between 50 and 75 microns. Ten production standard colours are now available. Special colours to British Standard or RAL colour range available upon request. Panels can also be supplied in Plastisol or Stelvelite finishes to the panel skin.

DOORS

Either steel or timber doors can be used with the system, and standard size doors match the panel module width exactly, thus enabling doors to be inter-changed or inserted in any standard 900mm panel width.

Special configured doors can also be supplied to individual requirements. Cellular or solid core timber veneered doors can be supplied in either steel or matching timber frames, and full height doors and matching overpanels are also available. Glazed apertures can be incorporated if required.

IRONMONGERY

Standard ironmongery is available but locks, lever handles and knobsets of choice may be specified. A variety of surface mounted door closers and panic exit hardware is available.

GLAZED PANELS

Panels can be partially or totally glazed, in single or double glazed arrangement, and as appropriate fire regulation requirements.

WEIGHT

18kg/sq metre average.

SPECIAL PANEL DETAILS

Panels can be custom manufactured to allow for on site cut-out details or any special display requirements.

SOUND

Partitions have been tested to BS5821 between 500 and 3150 Hz with average sound reduction indices as follows: all steel solid panels 35.1 dB; steel and double glazed panels 33.7 dB; steel and single glazed panels 26.8 dB.

THERMAL TRANSMISSION

Solid panels — U. value (nominal)

0.778 W/M²°C.

Single glazed — U. value (nominal)

3.45 W/M²°C.

Double glazed — U. value (nominal)

1.92 W/M²°C.

STRUCTURAL/MECHANICAL

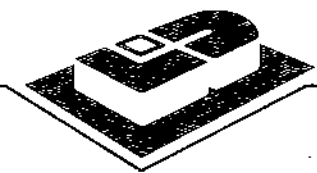
Reference can be made to BS5234-1975 and Master range of partitioning is non load bearing and has been designed to withstand typical loadings and impacts as would be expected in normal use, without incurring damage or structural failure.

MAINTENANCE

The partitioning is easily maintained, and all surfaces should be periodically cleaned with a damp cloth and mild detergent solution, wiping dry afterwards.

L & D TECHNICAL SUPPORT

A complete technical, computer aided design, and sales service is provided with a national coverage of Distribution and Installation centres throughout the UK and Ireland.



L & D PARTITIONING SYSTEMS LTD

Oxleasow Road, East Moons Moat, Redditch, Worcs B98 0RE, England
Telephone Redditch 0527 510015 Telex 335668 Fax 0527 517222



A member of the Associated British Companies PLC

Member of the Partitioning Industry Association



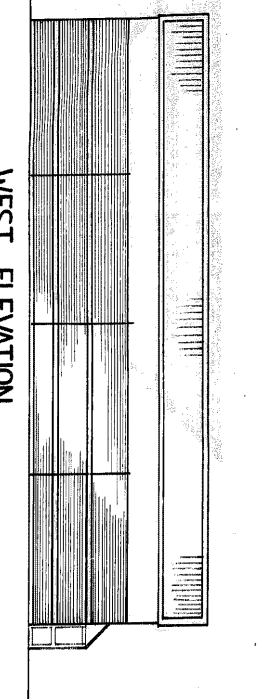
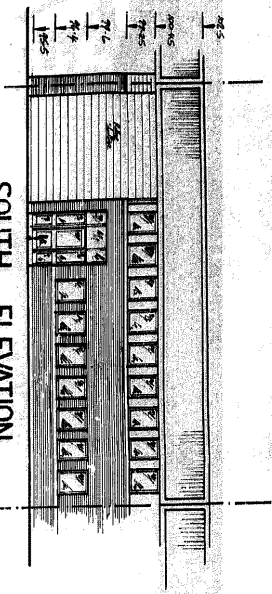
BS 5750/ISO 9002
No. 03994

Due to continuous product innovation L & D reserve the right to change product specification without due notice.



SOUTH ELEVATION

WEST ELEVATION



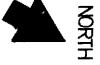
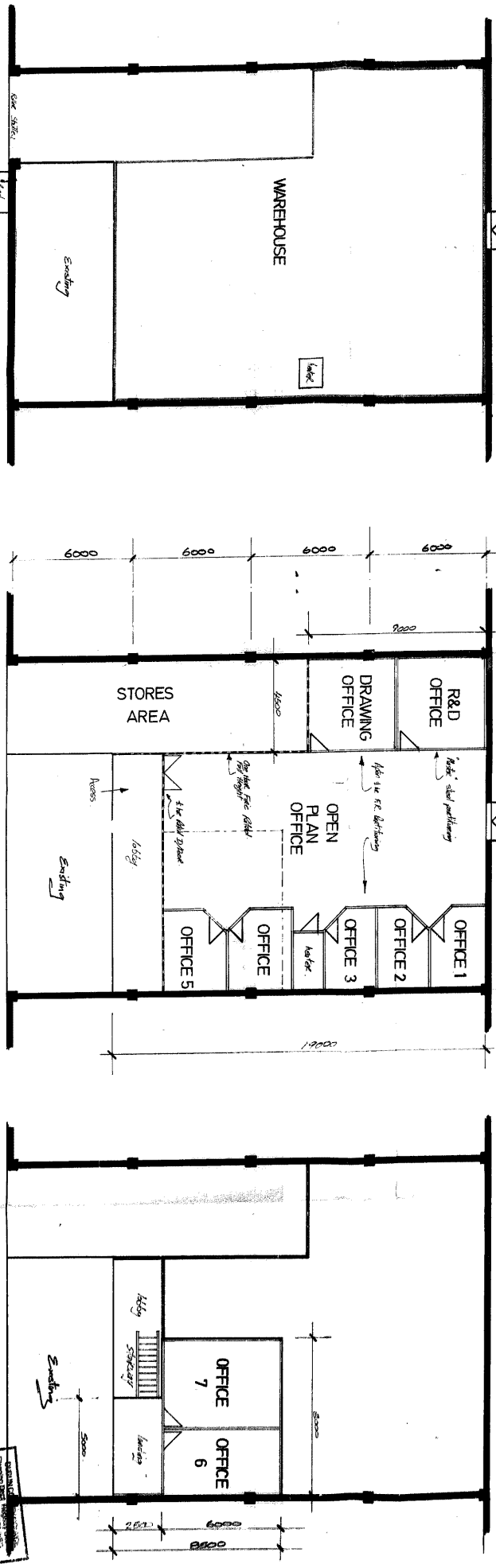
EXIT

EXIT

EXISTING GROUND FLOOR

PROPOSED GROUND FLOOR

PROPOSED MEZZANINE



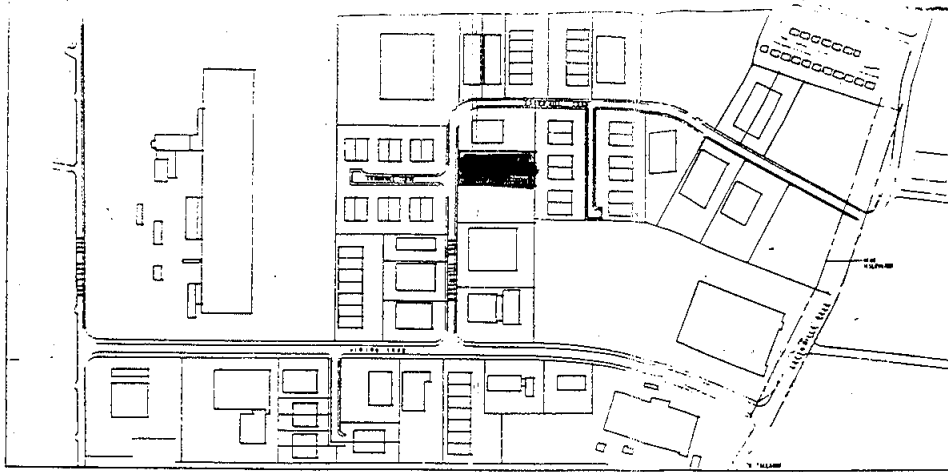
Consultant 19
U.S. Architecture
Company Name
Address
City, State, Zip
Phone
Fax
E-mail
Website

Project:
Owner:
Date:
Scale: **1:100**
Drawing No.: **10-0192-C**
Sheet No.: **1**

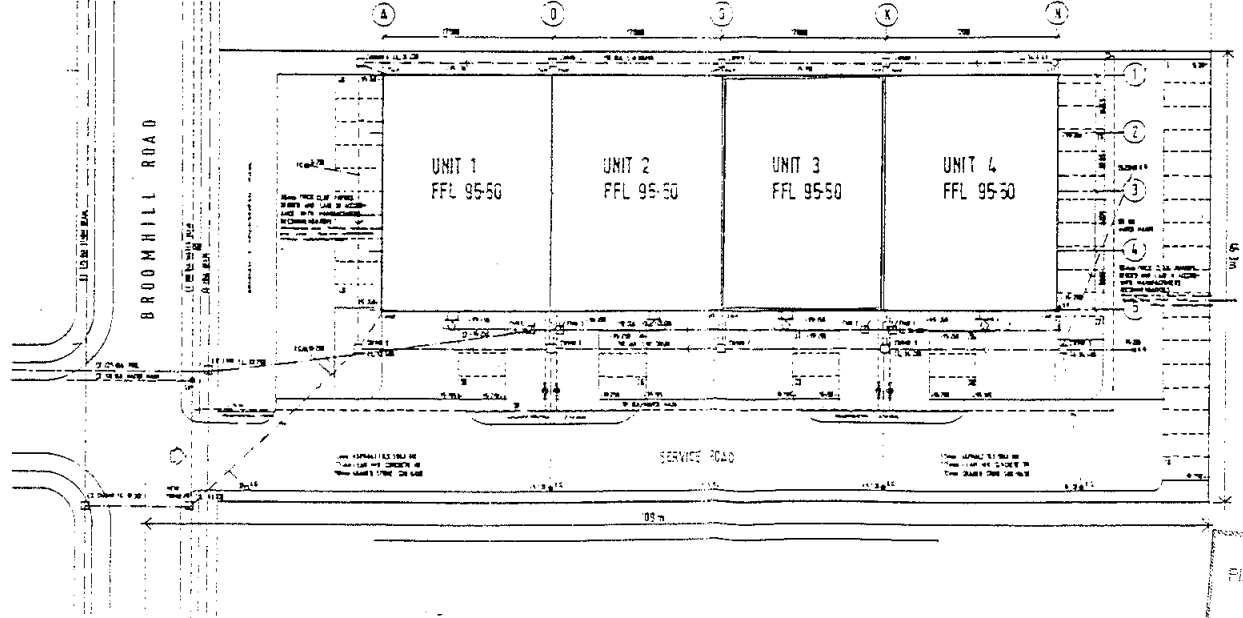
PROJECT: **PROPOSED MEZZANINE**
PREPARED BY: **ARCHITECTURE**
DATE: **1/13/14 HSE**
REVISION: **REV. NO. 1/13/14**

DEXION
13200 S. WILLOW AVE., SUITE 200
DENVER, CO 80227

When this installation is used in accordance with your premises use subject to the conditions of the existing structure capability for supporting the given loads.



SITE LOCATION



SITE PLAN

LEGEND

- SURFACE WATER DRAIN ————
- FOUL DRAIN ————
- WATER MAIN ————

DULLETT COUNTY
 Planning Department
 APPROVED
 1-11-2007
 REG No. 12210...



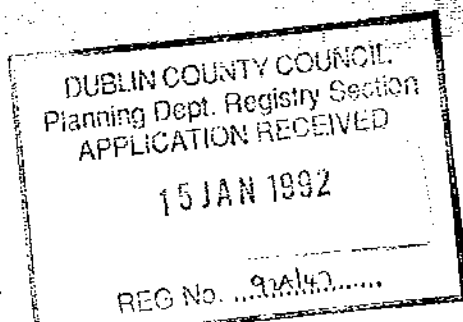
Stordex



**Mezzanine floors
with higher standards.**



Dexion Limited

Apton relocatable partitioning**GENERAL**

Introduction A system of high quality relocatable office partitioning which has been specially designed to keep on-site work to a minimum. Pre-formed windows and door frames, already fitted with glazing and factory-hung doors, are supplied with factory finished panels, to cut down installation labour and ensure consistent quality. The stove-enamelled *steel* framing provides exceptional rigidity and durability. Specific acoustic and fire resistance options are available.

Applications The wide range of standard panel configurations and finishes makes Apton partitioning suitable for office environments from basic offices on the shop floor to prestigious board rooms. Its well engineered design makes it particularly suitable for applications where changes or extensions to the initial layout are anticipated. Apton partitioning can also be used where irregular or unusually high ceilings have to be accommodated.

COMPOSITION, MANUFACTURE

Apton partitioning consists of a steel framework supporting plasterboard panels and modular windows and door units. The frame is made from vertical studs fixed at the base into a channel section floor track at 600 mm horizontal centres. Both studs and floor track are made from galvanised, rolled steel. Tops of studs are attached with special locators to stove enamelled steel ceiling track, which is also used at wall abutments. Galvanised steel channel section transoms are fitted between studs, above doors, above and below windows and where a transom line is to be featured on solid panels.

Window and door frames are of prefabricated steel. Windows are pre-glazed with trims fitted over the beads, and doors are pre-hung on lift-off butts. The plasterboard factory-finished panels are fitted with their lower edges resting in the L section pvc skirting. Top edges are located within ceiling track. Clamping strips fix panels and window and door frames hack into the framework with self tapping screws passing between panels into the framework, and clip-on pvc cover strips complete the structure.

Customer:- Aritech Itec
Site :- Broomhill DUBLIN.

Ref:- 5476

<<<<PLEASE NOTE>>>>

Where the sign "" is used in these calculations it signifies "TO THE POWER OF"
For example Y³ is equal to Y cubed.

1. DESIGN LOADING

Imposed load 4.5 kN/sq.m (from BS 6399 : Part 1 : 1984)

Dead load 0.5 kN/sq.m

2. FLOOR DECKING DESIGN

Try 15mm Finnish birch plywood

Section modulus per metre width = 34100 mm³
Permissible bending stress = 16.90 N/sq.mm

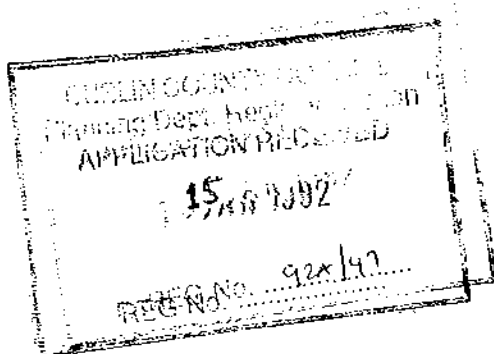
Joist centres = 525.0 mm

Design for IMPOSED load only
Load per metre width W = 4.5 x 1.0 x 0.525 = 2.362 kN

$$\text{Bending Moment} \frac{W.L}{8} = \frac{2362 \times 525.0}{8} = 155039 \text{ N.mm}$$

$$\text{Actual stress} \frac{M}{Z} = \frac{155039}{34100} = 4.55 \text{ N/sq.mm}$$

Actual stress = 4.55 < Permissible stress = 16.90



Customer:- Aritech Itec
Site :- Broomhill DUBLIN

Ref:- 5476

3. STEEL JOIST (MARK J1) DESIGN

Design to BS 5950 : Part 5 : 1987
Joist type:- 165mm Standard Joist
Yield strength of steel $Y_s = 300$ N/sq.mm
Design strength of steel $p_y = 300$ N/sq.mm

Section properties taking account of the effective widths of flange and lip in compression.

$I_{xx} = 2153800$ mm⁴ ; $Z_{xx} = 26107$ mm³ : Area = 550 mm²
 $r_{yy} = 17.6$ mm : Depth D = 165 mm : Thickness t = 2.0 mm

Joist centres = 375.0 mm Joist span = 3000.0 mm

Load factors used are from BS 5950,
for IMPOSED load = 1.6
for DEAD load = 1.4

(a) Bending limit

$$p_c = \left[1.13 - 0.0019 \times \frac{D (Y_s)^{0.5}}{t (280)} \right] \times p_y \leq p_y$$

Limiting compressive stress in a flat web $p_c = 290.3$ N/sq.mm

Factored TOTAL joist loading
 $v = (4.5 \times 1.6 + 0.5 \times 1.4) \times 0.375 = 2.963$ kN/m

$$\text{Bending Moment } \frac{vL^2}{8} = \frac{2.963 \times 3000.0^2}{8} = 3332810 \text{ N.mm}$$

$$\text{Factored Stress } \frac{M}{Z_{xx}} = \frac{3332810}{26107} = 127.7 \text{ N/sq.mm}$$

Factored stress = 127.7 < Design stress = 290.3

Customer:- Aritech Itec
Site :- Broomhill DUBLIN

Ref:- 5476

3. STEEL JOIST (MARK J1) DESIGN (Continued)

(b) Deflection limit

The maximum allowable deflection due to unfactored IMPOSED load is SPAN/200.

$$\text{IMPOSED joist loading } w = 4.5 \times 0.375 = 1.688 \text{ kN/m}$$

$$\text{Deflection } \frac{5wL^4}{384EI} = \frac{5 \times 1.688 \times 3000.0^4}{384 \times 205000 \times 2153800} = 4.031 \text{ mm}$$

$$\text{Permissible deflection} = \text{SPAN}/200 = 3000.0/200 = 15.000\text{mm}$$

(c) Web crushing limit

Web crushing not critical, as joist is web fixed.

See Appendix for detailed calculations.

(d) Maximum shear stress limit

$$pq = 0.7py = 0.7 \times 300.0 = 210.0 \text{ N/sq.mm}$$

$$Pq = \frac{2.pq.I_{xx}.t}{A.y} = \frac{2 \times 210.0 \times 2153800 \times 2.0}{550 \times 56.7} = 58.015 \text{ kN}$$

$$\text{Factored end load} = wL/2 = 4.444 \text{ kN}$$

(e) Average shear stress limit

p_v is the minimum of $0.6py$ or $(1000t/D)^2$

Therefore, $p_v = 146.9 \text{ N/sq.mm}$

$$P_v = p_v.D.t = 146.9 \times 165 \times 2.0 = 48.485 \text{ kN}$$

$$\text{Factored end load} = wL/2 = 4.444 \text{ kN}$$

Customer:- Aritech Itec
Site :- Broomhill DUBLIN

Ref:- 5476

4. STEEL JOIST (MARK J2) DESIGN

Design to BS 5950 : Part 5 : 1987
Joist type:- 178mm Heavy Joist
Yield strength of steel $Y_s = 300$ N/sq.mm
Design strength of steel $p_y = 300$ N/sq.mm

Section properties taking account of the effective widths of flange and lip in compression.

$I_{xx} = 3199590$ mm⁴ : $Z_{xx} = 35950$ mm³ : Area = 717 mm²
 $r_{yy} = 17.5$ mm : Depth D = 178 mm : Thickness t = 2.5 mm

Joist centres = 375.0 mm Joist span = 4800.0 mm

Load factors used are from BS 5950,
for IMPOSED load = 1.6
for DEAD load = 1.4

(a) Bending limit

$$p_c = \left[\begin{array}{l} D (Y_s)^{0.5} \\ 1.13 - 0.0019 \times \frac{D (Y_s)^{0.5}}{t (280)} \end{array} \right] \times p_y \leq p_y$$

Limiting compressive stress in a flat web $p_c = 297.0$ N/sq.mm

Factored TOTAL joist loading
 $w = (4.5 \times 1.6 + 0.5 \times 1.4) \times 0.375 = 2.963$ kN/m

$$\text{Bending Moment } \frac{wL^2}{8} = \frac{2.963 \times 4800.0^2}{8} = 8532000 \text{ N.mm}$$

$$\text{Factored Stress } \frac{M}{Z_{xx}} = \frac{8532000}{35950} = 237.3 \text{ N/sq.mm}$$

Factored stress = 237.3 < Design stress = 297.0

Customer:- Aritech Itec
Site :- Broomhill DUBLIN

Ref:- 5476

4. STEEL JOIST (MARK J2) DESIGN (Continued)

(b) Deflection limit

The maximum allowable deflection due to unfactored IMPOSED load is SPAN/200.

IMPOSED joist loading $w = 4.5 \times 0.375 = 1.688 \text{ kN/m}$

$$\text{Deflection} = \frac{5wL^4}{384EI} = \frac{5 \times 1.688 \times 4800.0^4}{384 \times 205000 \times 3199590} = 17.783 \text{ mm}$$

Permissible deflection = SPAN/200 = 4800.0/200 = 24.000mm

(c) Web crushing limit

Web crushing not critical, as joist is web fixed.

See Appendix for detailed calculations.

(d) Maximum shear stress limit

$p_q = 0.7p_y = 0.7 \times 300.0 = 210.0 \text{ N/sq.mm}$

$$P_q = \frac{2 \cdot p_q \cdot I_{xx} \cdot t}{A \cdot y} = \frac{2 \times 210.0 \times 3199590 \times 2.5}{717 \times 60.3} = 77.705 \text{ kN}$$

Factored end load = $wL/2 = 7.110 \text{ kN}$

(e) Average shear stress limit

p_v is the minimum of $0.6p_y$ or $(1000t/D)^2$

Therefore, $p_v = 180.0 \text{ N/sq.mm}$

$P_v = p_v \cdot D \cdot t = 180.0 \times 178 \times 2.5 = 80.100 \text{ kN}$

Factored end load = $wL/2 = 7.110 \text{ kN}$

Customer:- Aritech Itec
Site :- Broomhill DUBLIN

Ref:- 5476

5. MAIN BEAM (MARK B1) DESIGN

Design to BS 5950 : Part 1 : 1990
Beam type:- 305 x 127 x 37kg/m U.B. in grade 43 steel

Design strength of steel $p_y = 275 \text{ N/sq.mm}$

Section properties

$I_{xx} = 7160 \text{ cm}^4$: $Z_{xx} = 472 \text{ cm}^3$: $S_{xx} = 540 \text{ cm}^3$
 $r_{yy} = 2.67 \text{ cm}$: $u = 0.871$: $x = 29.6$

Main beam span, $L = 5900.0 \text{ mm}$
Main beam cantilever, $M = 150.0 \text{ mm}$

Load factors used are from BS 5950,
for IMPOSED load = 1.6
for DEAD load = 1.4

CHECK FOR LOAD ON MAIN SPAN ONLY

(a) Bending limit

Moment capacity, $M_C = p_y \cdot S_{xx} = 275 \times 540 = 149 \text{ kN.m}$

Factored TOTAL beam load
 $w = (4.5 \times 1.6 + 0.5 \times 1.4) \times 3.850 = 30.415 \text{ kN/m}$

Bending moment $\frac{wL^2}{8} = \frac{30.415 \times 5900.0^2}{8} = 132 \text{ kN.m}$

Bending moment $132 \text{ kN.m} < \text{Moment capacity } 149 \text{ kN.m}$

(b) Deflection limit

The maximum allowable deflection due to unfactored
IMPOSED load is $\text{SPAN}/200$.

IMPOSED beam loading, $w = 4.5 \times 3.850 = 17.325 \text{ kN/m}$

Deflection $\frac{5wL^4}{384EI} = \frac{5 \times 17.325 \times 5900.0^4}{384 \times 205000 \times 71600000} = 18.623 \text{ mm}$

Allowable deflection = $\text{SPAN}/200 = 5900.0/200 = 29.500 \text{ mm}$

Customer:- Aritech Itéc
Site :- Broomhill DUBLIN

Ref:- 5476

5. MAIN BEAM (MARK B1) DESIGN (Continued)

(c) Shear capacity limit

$$\text{Shear capacity, } P_v = 0.6 \cdot p_y \cdot A_v$$

where For rolled I sections, $A_v = tD$
 t = web thickness
 D = section depth

$$P_v = 0.6 \times 275 \times 7.2 \times 303.8 = 361 \text{ kN}$$

$$\text{Factored end load} = wL/2 = 90 \text{ kN}$$

(d) Web bearing limit

$$\text{Bearing capacity, } P_b = (b_1 + n_2)t \cdot p_{yw}$$

where b_1 = stiff bearing length
= 48.0mm (minimum)

n_2 = dispersion through flange
= $2.5(D-d)/2$

p_{yw} = design strength of web

$$P_b = [48.0 + 2.5(303.8 - 264.6)/2] \times 7.2 \times 275 = 192 \text{ kN}$$

$$\text{Factored end load} = wL/2 = 90 \text{ kN}$$

(e) Web buckling limit

$$\text{Web buckling capacity, } P_w = (b_1 + n_1)t \cdot p_c$$

where n_1 = dispersion at 45 degrees through
half section depth D

p_c = compressive strength using table 27(c)
for a slenderness ratio of $2.5d/t$

$$P_w = [48.0 + (303.8/2)] \times 7.2 \times 139 = 200 \text{ kN}$$

$$\text{Factored end load} = wL/2 = 90 \text{ kN}$$

Customer:- Aritech Itec
Site :- Broomhill DUBLIN

Ref:- 5476

5. MAIN BEAM (MARK B1) DESIGN (Continued)

CHECK FOR LOAD ON CANTILEVER ONLY

(f) Bending limit

The buckling resistance moment M_b , may be obtained from BS 5950 : Part 1 : 1990 Appendix B.

$$M_b = \frac{M_E.M_P}{O_b + (O_b^2 - M_E.M_P)^{0.5}}$$

Effective length L_E for the main span is $1.0L = 5900.0$ mm with a slenderness correction factor, $n = 0.770$
Effective length L_E for the cantilever is $0.7M = 105.0$ mm with a slenderness correction factor, $n = 0.870$

Therefore, buckling resistance moment, $M_b = 62$ kN.m

Factored TOTAL beam loading

$$w = (4.5 \times 1.6 + 0.5 \times 1.4) \times 3.850 = 30.415 \text{ kN/m}$$

$$\text{Bending moment} = \frac{wM^2}{2} = \frac{30.415 \times 150.0^2}{2} = 0 \text{ kN.m}$$

Factored moment = 0 kN.m < Buckling moment 62 kN.m

(g) Cantilever deflection limit

The maximum allowable deflection due to unfactored IMPOSED load is CANTILEVER/180.

IMPOSED beam loading, $w = 4.5 \times 3.850 = 17.325$ kN/m

$$\text{Deflection} = \frac{wLM^3}{24EI} \left(4 + \frac{3M}{L} \right)$$

$$\frac{17.325 \times 5900.0 \times 150.0^3}{24 \times 205000 \times 71600000} \left(4 + \frac{3 \times 150.0}{5900.0} \right) = 0.004 \text{ mm}$$

Permissible deflection = CANTILEVER/180 = $150.0/180 = 0.833$ mm

Customer:- Aritech Itec
Site :- Broomhill DUBLIN

Ref:- 5476

5. MAIN BEAM (MARK B1) DESIGN (Continued)

CHECK FOR LOAD ON MAIN SPAN AND ONE CANTILEVER

(h) Maximum support reaction

Factored TOTAL beam loading

$$w = (4.5 \times 1.6 + 0.5 \times 1.4) \times 3.850 = 30.415 \text{ kN/m}$$

$$R_a = \frac{w(L+M)^2}{2L} = \frac{30.415(5900.0 + 150.0)^2}{2 \times 5900.0} = 94 \text{ kN}$$

(i) Shear capacity limit

P_v is the same as for simply supported beam, $P_v = 361 \text{ kN}$

(j) Web bearing limit

Bearing capacity, $P_b = (b_1 + n_2)t.pyw$

where b_1 = stiff bearing length
= 100.0mm (minimum)

n_2 = dispersion through flange
= $2.5(D-d)/2 \times 2 = 2.5(D-d)$

pyw = design strength of web

$$P_b = [100.0 + 2.5(303.8 - 264.6)] \times 7.2 \times 275 = 392 \text{ kN}$$

(k) Web buckling limit

Web buckling capacity, $P_w = (b_1 + n_1)t.pc$

where n_1 = dispersion at 45 degrees through
half section depth D

pc = compressive strength using table 27(c)
for a slenderness ratio of $2.5d/t$

$$P_w = [100.0 + 303.8] \times 7.2 \times 139 = 404 \text{ kN}$$

Customer:- Aritech Itec
Site :- Broomhill DUBLIN

Ref:- 5476

6. INTERNAL COLUMN (MARK I1) DESIGN

Try a 100mm x 100mm x 5.0mm SHS

Design strength of steel $p_y = 275 \text{ N/sq.mm}$

Section properties

Square section therefore, $I_{xx} = I_{yy} = 283 \text{ cm}^4$

$Z_{xx} = Z_{yy} = 56.6 \text{ cm}^3$: $S_{xx} = S_{yy} = 67.1 \text{ cm}^3$

Area = 18.9 cm^2 : $r_{xx} = r_{yy} = 3.87 \text{ cm}$

Column height, $H = 2350 \text{ mm}$

Load factors used are from BS 5950,
for IMPOSED load = 1.6
for DEAD load = 1.4

CHECK COLUMN STRENGTH - SIMPLIFIED APPROACH

$$\frac{K H}{r_{xx}} = \frac{1.5 \times 2350}{38.7} = 91.09$$

Therefore, p_c from table 27(a) = 177.9 N/sq.mm

COLUMN LOADING

Total unfactored vertical load

$$W = (4.5 + 0.5) \times (11.358 + 2.760) = 70.588 \text{ kN}$$

Factored vertical load

$$W_A = (4.5 \times 1.6 + 0.5 \times 1.4) \times 11.358 = 89.724 \text{ kN}$$

$$W_B = (4.5 \times 1.6 + 0.5 \times 1.4) \times 2.760 = 21.804 \text{ kN}$$

As required by clause 2.4.2.3 of BS 5950, for sway stability, a notional horizontal of 0.5% of the dead and imposed vertical loads must be considered in the design of the columns and their base fixings.

Total floor area = 60.50 m^2

Total stiffness factor, EI/H^3 , for ALL columns, 357.624

Stiffness factor for THIS column, 44.703

Based on stiffness ratio, floor area for column = 7.563 m^2

Factored notional horizontal force

$$Q = (4.5 \times 1.6 + 0.5 \times 1.4) \times 7.563 \times 0.5\% = 0.299 \text{ kN}$$

Customer:- Aritech Itec
 Site :- Broomhill DUBLIN

Ref:- 5476

6. INTERNAL COLUMN (MARK I1) DESIGN (Continued)

(a) Fully loaded limits

Local capacity check

$$\frac{F}{Ag.py} + \frac{Mx}{Mcx} + \frac{My}{Mcy} = \frac{(WA+WB)}{Ag.py} + \frac{e(WA-WB)}{Sxx.py} + \frac{H.Q}{Syy.py} \leq 1.0$$

$$\frac{111528}{1890 \times 275.0} + \frac{50 \times 67920}{67100 \times 275.0} + \frac{299 \times 2350}{67100 \times 275.0} = 0.437$$

Overall buckling check

$$\frac{F}{Ag.pc} + \frac{m.Mx}{Mb} + \frac{m.My}{py.Zyy} = \frac{(WA+WB)}{Ag.pc} + \frac{1.0e(WA-WB)}{Sxx.py} + \frac{0.57H.Q}{Zyy.py} \leq 1.0$$

$$\frac{111528}{1890 \times 177.9} + \frac{1.00 \times 50 \times 67920}{67100 \times 275.0} + \frac{0.57 \times 299 \times 2350}{56600 \times 275.0} = 0.542$$

(b) Part loaded limits

Local capacity check

$$\frac{F}{Ag.py} + \frac{Mx}{Mcx} + \frac{My}{Mcy} = \frac{WA}{Ag.py} + \frac{e.WA}{Sxx.py} + \frac{H.Q}{Syy.py} \leq 1.0$$

$$\frac{89724}{1890 \times 275.0} + \frac{50 \times 89724}{67100 \times 275.0} + \frac{299 \times 2350}{67100 \times 275.0} = 0.454$$

Overall buckling check

$$\frac{F}{Ag.pc} + \frac{m.Mx}{Mb} + \frac{m.My}{py.Zyy} = \frac{WA}{Ag.pc} + \frac{1.0 e WA}{Sxx.py} + \frac{0.57 H Q}{Zyy.py} \leq 1.0$$

$$\frac{89724}{1890 \times 177.9} + \frac{1.00 \times 50 \times 89724}{67100 \times 275.0} + \frac{0.57 \times 299 \times 2350}{56600 \times 275.0} = 0.536$$

Customer:- Aritech Itec
Site :- Broomhill DUBLIN

Ref:- 5476

6. INTERNAL COLUMN (MARK I1) DESIGN (Continued)

CHECK COLUMN BASEPLATE STRENGTH

Baseplate size D = 250 mm : B = 250 mm : t = 20.0 mm

(c) Concentric forces

$$t = \left[\frac{2.5}{pyp} w(a^2 - 0.3b^2) \right]^{0.5}$$

where a = greater projection = 75.0 mm
b = lesser projection = 75.0 mm

$$w = \text{pressure} = \frac{(WA+WB)}{D \times B} = \frac{111528}{62500} = 1.78 \text{ N/sq.mm}$$

pyp = design strength of plate = 265 N/sq.mm

Required thickness = 8.1 mm < actual thickness = 20.0 mm

(d) Eccentric forces

Fully loaded column

P = WA + WB = 111.528 kN
M = e(WA-WB) + Q.H = 4098000 N.mm

$$ex = \frac{M}{P} = \frac{4098000}{111528} = 36.7 \text{ mm which is } < \frac{D}{6}$$

Maximum pressure at edge of plate, f1 = 3.358 N/sq.mm
Maximum pressure at edge of column, f3 = 2.414 N/sq.mm

$$\text{Applied moment} = \frac{f3 \cdot a^2}{6} + \frac{f1 \cdot a^2}{3} = 8559 \text{ N.mm}$$

$$\text{Limiting moment} = 1.2 \frac{pyp \cdot t^2}{6} = 21200 \text{ N.mm}$$

Customer:- Aritech Itec
Site :- Broomhill DUBLIN

Ref:- 5476

6. INTERNAL COLUMN (MARK I1) DESIGN (Continued)

Part loaded column

$$P = WA = 89.724 \text{ kN}$$
$$M = e.WA + Q.H = 5188200 \text{ N.mm}$$

$$e_x = \frac{M}{P} = \frac{5188200}{89724} = 57.8 \text{ mm which is } > \frac{D}{6}$$

Maximum pressure at edge of plate, $f_1 = 3.562 \text{ N/sq.mm}$
Maximum pressure at edge of column, $f_3 = 2.236 \text{ N/sq.mm}$

$$\text{Applied moment} = \frac{f_3.a^2}{6} + \frac{f_1.a^2}{3} = 8775 \text{ N.mm}$$

$$\text{Limiting moment} = 1.2 \frac{pyp.t^2}{6} = 21200 \text{ N.mm}$$

CHECK COLUMN DEFLECTION

COLUMN LOADING

Unfactored notional horizontal force
 $5.0 \times 7.563 \times 0.5\% = 0.189 \text{ kN}$

(e) Deflection limit

The maximum allowable deflection due to unfactored IMPOSED load is HEIGHT/300.

Taking a simplified approach, assume the column acts as a cantilever with force Q at one end.

$$\text{Deflection} = \frac{QH^3}{3EI} = \frac{189 \times 2350^3}{3 \times 205000 \times 2830000} = 1.410 \text{ mm}$$

$$\text{Allowable deflection} = \text{HEIGHT}/300 = 2350/300 = 7.833 \text{ mm}$$

Customer:- Aritech Itec
Site :- Broomhill DUBLIN

Ref:- 5476

7. EXTERNAL COLUMN (MARK E1) DESIGN

Try a 100mm x 100mm x 5.0mm SHS

Design strength of steel $p_y = 275 \text{ N/sq.mm}$

Section properties

Square section therefore, $I_{xx} = I_{yy} = 283 \text{ cm}^4$
 $Z_{xx} = Z_{yy} = 56.6 \text{ cm}^3$: $S_{xx} = S_{yy} = 67.1 \text{ cm}^3$
Area = 18.9 cm^2 : $r_{xx} = r_{yy} = 3.87 \text{ cm}$

Column height, $H = 2350 \text{ mm}$

Load factors used are from BS 5950,
for IMPOSED load = 1.6
for DEAD load = 1.4

CHECK COLUMN STRENGTH - SIMPLIFIED APPROACH

$$\frac{K H}{r_{xx}} = \frac{1.5 \times 2350}{38.7} = 91.09$$

Therefore, p_c from table 27(a) = 177.9 N/sq.mm

COLUMN LOADING

Total unfactored vertical load
 $W = (4.5 + 0.5) \times 11.942 = 59.711 \text{ kN}$

Factored vertical load
 $WF = (4.5 \times 1.6 + 0.5 \times 1.4) \times 11.942 = 94.344 \text{ kN}$
 $WP = (4.5 \times 1.6 + 0.5 \times 1.4) \times 11.358 = 89.724 \text{ kN}$

As required by clause 2.4.2.3 of BS 5950, for sway stability, a notional horizontal of 0.5% of the dead and imposed vertical loads must be considered in the design of the columns and their base fixings.

Total floor area = 60.50 m^2

Total stiffness factor, EI/H^3 , for ALL columns, 357.624
Stiffness factor for THIS column, 44.703
Based on stiffness ratio, floor area for column = 7.563 m^2

Factored notional horizontal force
 $Q = (4.5 \times 1.6 + 0.5 \times 1.4) \times 7.563 \times 0.5\% = 0.299 \text{ kN}$

Customer:- Aritech Itec
 Site :- Broomhill DUBLIN

Ref:- 5476

7. EXTERNAL COLUMN (MARK E1) DESIGN (Continued)

(a) Fully loaded limits

Local capacity check

$$\frac{F}{Ag.py} + \frac{Mx}{Mcx} + \frac{My}{Mcy} = \frac{WF}{Ag.py} + \frac{0}{Sxx.py} + \frac{H.Q}{Syy.py} \leq 1.0$$

$$\frac{94344}{1890 \times 275.0} + \frac{0}{67100 \times 275.0} + \frac{299 \times 2350}{67100 \times 275.0} = 0.220$$

Overall buckling check

$$\frac{F}{Ag.pc} + \frac{m.Mx}{Mb} + \frac{m.My}{py.Zyy} = \frac{WF}{Ag.pc} + \frac{0}{Sxx.py} + \frac{0.57H.Q}{Zyy.py} \leq 1.0$$

$$\frac{94344}{1890 \times 177.9} + \frac{0}{67100 \times 275.0} + \frac{0.57 \times 299 \times 2350}{56600 \times 275.0} = 0.306$$

(b) Part loaded limits

Local capacity check

$$\frac{F}{Ag.py} + \frac{Mx}{Mcx} + \frac{My}{Mcy} = \frac{WP}{Ag.py} + \frac{e.WP}{Sxx.py} + \frac{H.Q}{Syy.py} \leq 1.0$$

$$\frac{89724}{1890 \times 275.0} + \frac{50 \times 89724}{67100 \times 275.0} + \frac{299 \times 2350}{67100 \times 275.0} = 0.454$$

Overall buckling check

$$\frac{F}{Ag.pc} + \frac{m.Mx}{Mb} + \frac{m.My}{py.Zyy} = \frac{WP}{Ag.pc} + \frac{1.0 e WP}{Sxx.py} + \frac{0.57 H Q}{Zyy.py} \leq 1.0$$

$$\frac{89724}{1890 \times 177.9} + \frac{1.00 \times 50 \times 89724}{67100 \times 275.0} + \frac{0.57 \times 299 \times 2350}{56600 \times 275.0} = 0.536$$

Customer:- Aritech Itec
Site :- Broomhill DUBLIN

Ref:- 5476

7. EXTERNAL COLUMN (MARK B1) DESIGN (Continued)

CHECK COLUMN BASEPLATE STRENGTH

Baseplate size D = 250 mm : B = 250 mm : t = 20.0 mm

(c) Concentric forces

$$t = \frac{[2.5 \sqrt{w(a^2 - 0.3b^2)}]}{pyp} \times 0.5$$

where a = greater projection = 75.0 mm
b = lesser projection = 75.0 mm

$$w = \text{pressure} = \frac{WF}{DxB} = \frac{94344}{62500} = 1.51 \text{ N/sq.mm}$$

pyp = design strength of plate = 265 N/sq.mm

Required thickness = 7.5 mm < actual thickness = 20.0 mm

(d) Eccentric forces

Fully loaded column

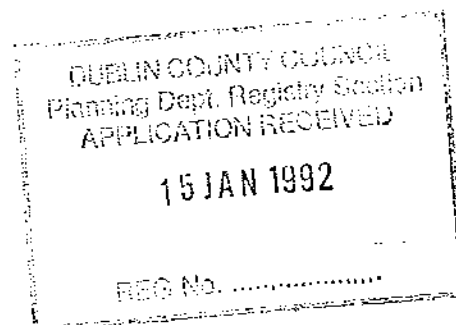
P = WF = 94.344 kN
M = Q.H = 701989 N.mm

$$ex = \frac{M}{P} = \frac{701989}{94344} = 7.4 \text{ mm which is } < \frac{D}{6}$$

Maximum pressure at edge of plate, f1 = 1.779 N/sq.mm
Maximum pressure at edge of column, f3 = 1.617 N/sq.mm

$$\text{Applied moment} = \frac{f3 \cdot a^2}{6} + \frac{f1 \cdot a^2}{3} = 4852 \text{ N.mm}$$

$$\text{Limiting moment} = 1.2 \frac{pyp \cdot t^2}{6} = 21200 \text{ N.mm}$$



Customer:- Aritech Itec
Site :- Broomhill DUBLIN

Ref:- 5476

7. EXTERNAL COLUMN (MARK E1) DESIGN (Continued)

Part loaded column

$$P = WP = 89.724 \text{ kN}$$
$$M = e.WP + Q.H = 5188200 \text{ N.mm}$$

$$e_x = \frac{M}{P} = \frac{5188200}{89724} = 57.8 \text{ mm which is } > \frac{D}{6}$$

Maximum pressure at edge of plate, $f_1 = 3.562 \text{ N/sq.mm}$
Maximum pressure at edge of column, $f_3 = 2.236 \text{ N/sq.mm}$

$$\text{Applied moment} = \frac{f_3.a^2}{6} + \frac{f_1.a^2}{3} = 8775 \text{ N.mm}$$

$$\text{Limiting moment} = 1.2 \frac{pyp.t^2}{6} = 21200 \text{ N.mm}$$

CHECK COLUMN DEFLECTION

COLUMN LOADING

Unfactored notional horizontal force
 $5.0 \times 7.563 \times 0.5\% = 0.189 \text{ kN}$

(e) Deflection limit

The maximum allowable deflection due to unfactored IMPOSED load is HEIGHT/300.

Taking a simplified approach, assume the column acts as a cantilever with force Q at one end.

$$\text{Deflection} = \frac{QH^3}{3EI} = \frac{189 \times 2350^3}{3 \times 205000 \times 2830000} = 1.410 \text{ mm}$$

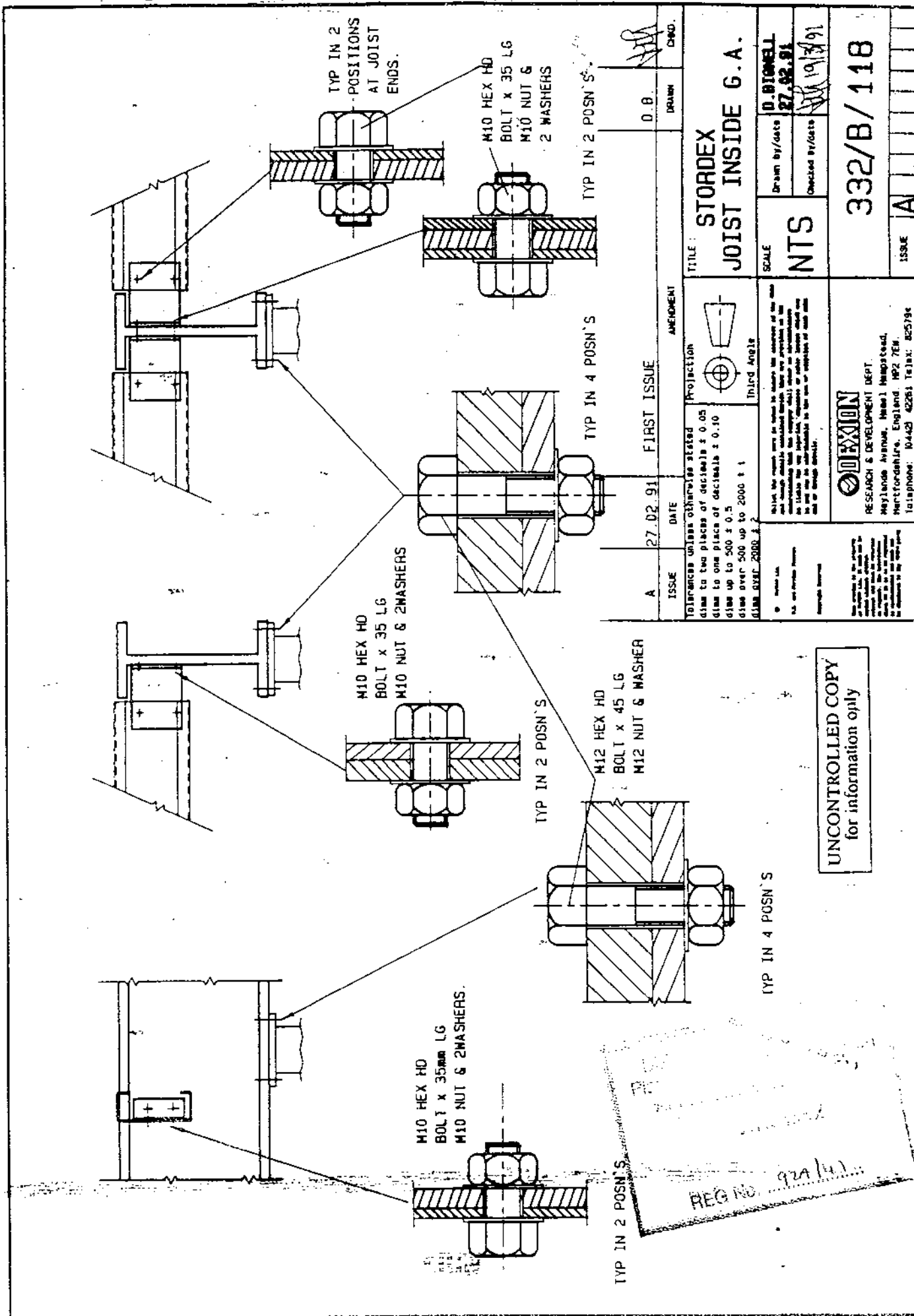
$$\text{Allowable deflection} = \text{HEIGHT}/300 = 2350/300 = 7.833 \text{ mm}$$

APPROVED FOR CONSTRUCTION
BY: [Signature]
DATE: 10 JAN 1997

92A102

REG.

92A102



ISSUE	DATE	AMENDMENT	TITLE
A	27.02.91	FIRST ISSUE	STORDEX JOIST INSIDE G.A.
TOLERANCES UNLESS OTHERWISE STATED DIM TO TWO PLACES OF DECIMALS ± 0.05 DIM TO ONE PLACE OF DECIMALS ± 0.10 DIM UP TO 500 ± 0.5 DIM OVER 500 UP TO 2000 ± 1 DIM OVER 2000 ± 2			SCALE NTS
DIMS OVER 2000 ± 2 THIRD ANGLE Projection			DRAWN BY/DATE D.B. 27.02.91
UNCONTROLLED COPY for information only			CHECKED BY/DATE 19/3/91
UNCONTROLLED COPY for information only			ISSUE 332/B/118
DEXION RESEARCH & DEVELOPMENT DEPT. MAYLEND AVENUE, HEMEL HEMPSTEAD, HERTFORDSHIRE, ENGLAND HP2 7EM. TELEPHONE: 04423 42261 FAX: 825794			

REG NO. 92A/41

C l i e n t :

A R I T E C H I T E C .
BROOMHILL TERRACE
BROOMHILL ROAD
DUBLIN 24.

P r o p o s a l :

INTERNAL OFFICE DEVELOPMENT
AND ANCILLARY SERVICES.

P r e p a r e d B y :

DEXION IRELAND LIMITED
15, CLONDALKIN IND. EST.
CLONDALKIN.
DUBLIN 22.

C o n t a c t :

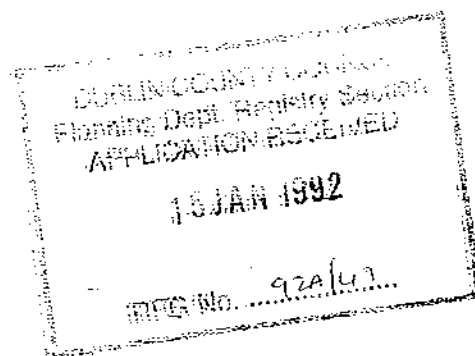
M r . J o h n C u m m i n s

P h o n e :

01-571016

F a x :

01-571236.



1

PROPOSAL

The proposal involves the installation of ground floor and mezzanine level partitioning in the existing warehouse area.

This project involves the installation of a mezzanine floor as shown on the drawing, complete with access stairway.

Under the mezzanine level the ground floor area of the warehouse will be fitted with office partitioning as per the layout drawing.

A suspended ceiling will be placed over the entire area and underneath the mezzanine level.

On the upper mezzanine two offices will be formed as per our layout drawing, a suspended ceiling will be fitted to this area and to the lobby area as shown on the drawing.

All offices will be fitted with electrical power supply as detailed below and lighting, again as detailed below.

Generally, all partitioning will be constructed from the Apton Demountable Partitioning Range, with the exception of the R&D office area, which will be constructed from Double Skin Steel Partitioning, Master Range or similarly approved.

Partitions will be all solid throughout, all doors will be half hour fire rated, complete with door closers intumescent door seals.

Adjacent to each doorway, a vision panel window will be fitted, this will measure 2.1 mtrs high X 300mm wide and will be constructed from Apton standard components. Glass will be 6mm georgian wire polished plate.

All doors will be fitted with union cylinder door locks with panic bolts and union door furniture in aluminium.

1. PARTITIONING.

Partitioning generally throughout the installation will be Apton Demountable Steel Stud System. These partitions will be built in accordance with the manufacturers instructions to achieve to $\frac{1}{2}$ hour fire ratings throughout. The exception to this would be the full height 1 hour fire rated partition as shown on the drawing.

2. R. & D. OFFICE.

The R & D Office will be formed from Master Range Double Skin Steel Partitioning. This will be built in accordance with manufactures instructions and will be all solid in elevation, complete with one access doorway half hour fire rated.

3. MEZZANINE FLOOR.

The Mezzanine floor will be constructed from the Stordex Range. The structure will have a design loading of 5 kn/m^2 and will be accessed by one access steel staircase, built in accordance with design calculations and building regulations. Support columns to the Mezzanine floor will be clad to achieve 1 hour fire rating.

4. SUSPENDED CEILING.

Suspended Ceilings will be fitted on the upper Mezzanine Offices, the Stairway Lobby and the Warehouse area over the new offices. Ceilings will be 600 X 600, exposed grid type, with fissured white ceiling panels. Ceilings will be installed to manufacturers instructions and will be constructed using the Armstrong System or similarly approved.

5. CARPET.

Carpet and underlay will be fitted throughout the new ground floor offices and mezzanine level offices complete with underlay to clients specification.

6. LIGHTING.

Lighting will be provided throughout using 600 X 1200 modular light fittings. Fittings will be 4 tube floresant type, fitted with presmatic defusers.

7. ELECTRICAL POWER SUPPLY.

Each office will be fitted with 4, 13 amp, sockets and one switch spur heater outlet, with a 1.5 kw rating. All electical wiring will conform to the statuary regulations. Electrical heaters will be Dimplex or similarly approved, 1 kw heaters fitted with thermostat. Heaters will be supplied and fitted by client at a later stage.

8. ELECTRICAL INSTALLATION.

Electrical installation will be complete with main supply swith, all necessary MCB's labelled clearly and fitted in an enclosure in the specified area.

APTON PARTITIONING SPECIFICATION

1. CONSTRUCTION

The partition shall be erected in standard 1200mm modules with make-up pieces as necessary. It shall comprise of ceiling, floor and wall track with vertical studs clad on both sides with 12.7mm plasterboard held in position by steel fixing strips and plastic cover trims. Pre-formed door and window frames fit into the basic framework as required.

2. FRAMING

The framework shall consist of 0.6mm zinc plated cold rolled steel floor track and studs, and 0.6mm mild steel stove enamelled (colour Ral 8019) ceiling track and wall track overall 86mm wide x 33mm deep with outer flanges 23mm deep. The floor track is screwed to the floor by the appropriate screws. The wall and ceiling tracks are similarly screwed to their abutments. The studs fit into the floor track and are pop-rivettted to same. A connector unit is fitted to the top of the stud and clipped and pop-rivettted onto the ceiling track. Studs supplied in standard lengths of 3000mm are fixed at 600mm centres to enable services to be passed within. Where the window frames and door frames are to be fitted, transoms are pop-rivettted to studs with transoms spanning 1200mm.

3. PANELS

The steel framework shall be clad on both sides with 12.7mm square edged plasterboard sheet conforming to BS 1230, Gypsum Plasterboard, Part 2, 1970, and consisting of an aerated Gypsum core encased in and firmly bonded to specially prepared durable paper liners. The sheets 1200mm x 3000mm high are cut on site to required heights.

The plasterboard panels shall be fitted under the lip edge of the ceiling and wall tracks and rest against the floor track. The panels are held in position by vertical and horizontal steel section strips fixed with self tapping screws at 300mm centres to the studs and transoms. Where horizontal strips are fitted along the transom, these shall be cut to fit below and above the horizontal strip. A PVC cover strip shall be clipped into the steel strip to cover screw heads and act as a feature.

The bottom edge of the plasterboard panels shall be encased in a 'J' shaped PVC skirting, 48mm high, which clips onto the plasterboard edge. The skirting shall be supplied in lengths of 3000mm and cut to length on site.

4. DECOR

The plasterboard panels shall be delivered to site pre-decorated. The fabric shall be stuck to the board with an approved adhesive.

5. DOORS

The doors shall be 2040mm x 826mm x 40mm hollow core, or solid core in fire rated partitions, painted for sapele veneer finish, and shall be pre-hung on 3 No. standard hinges, screwed into the threaded sockets incorporated in the doorframe.

The frame shall be constructed of .8mm zinc plated steel channel section enamelled, colour Brown RAL 8019. The frame shall be fitted into the stud framework and the stiles shall be located by means of 2 floor fixed brackets.

The doors shall be fitted with a standard SAA level handle, mortice latch, and lock, and a striking plate shall be mounted separately on the doorframe (or type specified).

6. WINDOWS

The window frames shall be constructed of 0.8mm thick electrically zinc plated steel sheet enamelled, colour Brown. The frames shall be pre-glazed with the required glass which is held in position by steel glazing beads and glazing gasket.

Where fire rated partition is required 6mm Georgian Wired Glass to be used.

7. TRIMS

The retaining strips shall be steel with Brown PVC snap-on cover strip. Internal and external corners shall be in steel, stove enamelled, colour Brown.

8. ACCUSTIC INFILL

Mineral wool slabs (density 20kg/m³) x 50mm thick, shall be inserted inside the partition in order to further improve the acoustic insulation properties, or where half hour fire rating is required.

9. ACOUSTIC RATING

The partitioning will give the following sound attenuation over a range of 400-2500 Hz under test conditions, defined as a mean sound reduction index (to BS 2750).

	(dB)
a) Solid Panel - without acoustic infill	- 35 - 36
b) Solid Panel - with acoustic infill	- 41 - 42
c) Centre Glazed Panel	- 26 - 27
d) Transom Glazed Panel	- 27 - 28
e) Half Glazed Panel	- 24 - 26

10. FIRE RATING

The system does provide a half hour fire rating as defined in Building Regulations, when acoustic infill is used, in conjunction with solid core doors and Georgian Wired Glass.

11. SPREAD-OF-FLAME

The system without the decorative fabric finish complies with Class 'O' Spread of Flame rating as defined in BS 467, Part 4.

The standard decorative fabric applied to the plasterboard panels complies with Class 'O' Spread of Flame.

PRODUCT SPECIFICATION

APTON PARTITIONING

Partitions will be installed as per the layout drawing and fire rated as stated on the drawing.

STORDEX RAISED STORAGE AREA

GENERAL INFORMATION

Stordex Raised Storage Areas are designed with the use of the latest computer technology. Design and manufacture conforms to British Standards 449:Part 2:1969 'Specification for the Use of Structural Steel in Building', and Addendum No.1 (April 1975) 'Specification for the Use of Cold Formed Steel Sections in Building'.

Structures built in this way satisfy the requirements of Section D8 of the main Building Regulations 1976 relating to structural integrity. The main beams are manufactured to BS4:Part 1:1972 'Structural Steel Sections - Hot Rolled Sections'. The columns are manufactured in compliance with BS4848:Part 2 - Hollow Sections using material Grade 43 to BS4360:1979 'Specification for Weldable Structural Steel'.

The stairways and handrails comply with Building Regulations (Section H), and relate to the British Standard Code of Practice. The handrail, in fact, meets the higher standard required for public buildings 740 N/M whilst the handrailing generally offered by suppliers of this type of installation only meets the lower private use standard.

The steel stair treads have a hard-wearing non-slip finish as standard. The general safety feature of the Cat Ladder are based on the recommendations of BS4211.

All bolts in the structural connections are to BS3692:1976 'ISO Metric Precision Hexagon Bolts', and have a bright Zinc passivated finish.

The Stordex system comprises steel joists sections laid over structural steel main beams. These beams are supported by R.H.S. steel stanchions. Once the columns have been levelled, each one is secured to the concrete floor by four 12mm x 125mm high tensiles steel studs, resin bonded to the concrete.

Any air space below the baseplate, caused by an uneven floor, is then filled with a pourable cementitious structural grout to ensure that all loads transferred to the floor are spread over a consistent predetermined area.

Stordex is completely free-standing and imposes no additional load on the above ground structure of the building.

In addition to being functional, the Stordex system has been designed to be good looking and pleasing in appearance. All steel components are finished with a smooth, hard-wearing paint finish prior to leaving our factory therefore eliminating site finishing after installation. A number of the components, including the stairways, handrailing, joists and stanchions are stove enamelled to British Standards for a long maintenance-free operational life.

Floor Material: 15mm Finly Birch Plywood.
Joists: 165 std duty/178 E/Heavy Duty.
Main Beam: 305 X 127 X 37 U.S.
Columns: 100 X 100 X 5 RHS
Design Load: 5.0 kn/per mtr sq.
Client: Aritech Itec.

Dublin County Council Comhairle Chontae Atha Cliath Planning Department

Building Control Department,
Liffey House,
Tara Street,
Dublin 1.
Telephone:773066



Bloc 2, Ionad Bheatha na hEireann,
Bloc 2, Irish Life Centre,
Sraid na Mainistreach Iacht,
Lower Abbey Street,
Baile Atha Cliath 1.
Dublin 1.
Telephone (01) 724755
Fax (01) 724896

Register Reference : 92A/0047

Date : 16th January 1992

LOCAL GOVERNMENT (PLANNING AND DEVELOPMENT) ACTS, 1963 TO 1990

Dear Sir/Madam,

DEVELOPMENT : Internal office development and ancillary services

LOCATION : Unit 3, Broomhill Terrace, Tallaght Industrial Estate
Co. Dublin

APPLICANT : Aritech Ireland

APP. TYPE : PERMISSION/BUILDING BYE-LAW APPROVAL

With reference to the above, I acknowledge receipt of your application received on 15th January 1992.

Yours faithfully,

.....

FOR PRINCIPAL OFFICER

Dexion Ireland Limited,
Unit 15, Clondalkin Industrial
Estate,
Clondalkin,
Dublin 22



PLEASE READ INSTRUCTIONS AT BACK BEFORE COMPLETING FORM. ALL QUESTIONS MUST BE ANSWERED.

1. Application for Permission Outline Permission Approval Place in appropriate box.
Approval should be sought only where an outline permission was previously granted. Outline permission may not be sought for the retention of structures or continuances of uses.

2. Postal address of site or building ARITECH IRELAND
(If none, give description sufficient to identify) Unit 3, Broomhill Terrace, Tallaght Ind. Est. Tallaght, Dublin 24

3. Name of applicant (Principal not Agent) Aritech Ireland
Address Unit 3, Broomhill Terrace, Tallaght, Dublin 24 Tel. No. 521055

4. Name and address of Dexion Ireland Limited, Unit 15, Clondalkin Ind. Est.
person or firm responsible for preparation of drawings Clondalkin, Dublin 22. Tel. No. 571016

5. Name and address to which Dexion Ireland Limited.
notifications should be sent Unit 15, Clondalkin Ind Est. Clondalkin, Dublin 22

6. Brief description of proposed development Internal Office Development.

7. Method of drainage N/A 8. Source of Water Supply N/A

9. In the case of any building or buildings to be retained on site, please state:
(a) Present use of each floor or use when last used. Warehouse Space
(b) Proposed use of each floor Administration Offices

10. Does the proposal involve demolition, partial demolition or change of use of any habitable house or part thereof? NO

11. (a) Area of Site N/A Sq. m.
(b) Floor area of proposed development 370 Sq. m.
(c) Floor area of buildings proposed to be retained within site N/A Sq. m.

12. State applicant's legal interest or estate in site (i.e. freehold, leasehold, etc.) LEASEHOLD

13. Are you now applying also for an approval under the Building Bye Laws?
Yes No Place in appropriate box.

14. Please state the extent to which the Draft Building Regulations have been taken in account in your proposal:

15. List of applications: DUBLIN 24 - Planning permission sought from Dublin County Council for internal office development and ancillary services at Unit 3, Broomhill Terrace, Tallaght Industrial Estate, Co. Dublin for Aritech Ireland.
4 X Specification Ref: 1319
4 X Drawing No: 10 - 0192 - C.
4 X Drawing No: 11 - 0192 - C.

16. Gross floor space of proposed development (See back) N/A Sq. m.
No of dwellings proposed (if any) N/A Class(es) of Development N/A
Fee Payable £ 647.50 Basis of Calculation 370 X £1.75
If a reduced fee is tendered details of previous relevant payment should be given

Signature of Applicant (or his Agent) [Signature] Date 13/1/92

Application Type P/B FOR OFFICE USE ONLY
Register Reference 92A/0047
Amount Received £ 21-8 2.20.4.12 15/1
Receipt No
Date

*Shil
Lines
11/1/92*

*647.50
N 54372
15/1/92*

CHAIRLE CHONTAE ÁTHA CLIAITH

RECEIPT CODE

PAID BY — DUBLIN COUNTY COUNCIL

46/49 UPPER O'CONNELL STREET

DUBLIN 1.

18000 of this receipt is not an
acknowledgement that the fee
tendered is the prescribed application
fee. ✓

54372

LIT.

M.O.

B.L.

CASH

CHEQUE

£ 647.50

Received this 15th day of June 1992

from Denon Ireland Ltd
Unit 15 Clonsilla Industrial Estate

the sum of six hundred and 47 pence

paid by seven

for the purpose of being

application of Unit 3 Broomhill Estate, Tallaght, Dublin 24

Michael O'Leary Cashier

S. CAREY
Principal Officer

